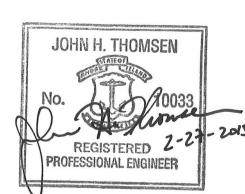


NOTE: RENDERED STRUCTURAL VIEW FOR GENERAL PROJECT SCOPE ONLY. NOT ALL STRUCTURAL ELEMENTS ARE SHOWN.

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Number	Date	Revision

one quart

one eighth inch = one foot

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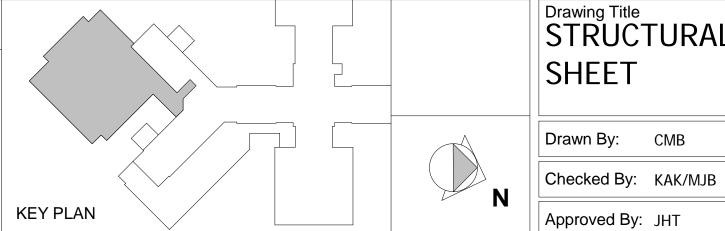
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41 Seyon Street
Waltham, MA 02453 MEP / FP
Thompson Consulting, Inc. 525 Mill Street Marion, MA 02738

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COST D.G. Jones 3 Baldwin Green Common Suite 202 Woburn, MA 01801 T: 781.932.3131 www.thompson-consulting.com www.dgjonesboston.com

ARCHITECT morris switzer
environments for health 11 Drydock Ave, Suite 2060 Boston, MA 02210 T: 617.772.0260 F: 617.772.0261 www.morrisswitzer.com



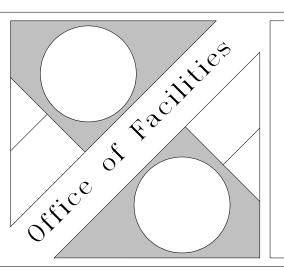
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REPLACE/RE DEFICIENT IC	
Building #	Scale
Location	

PROVIDENCE, RI

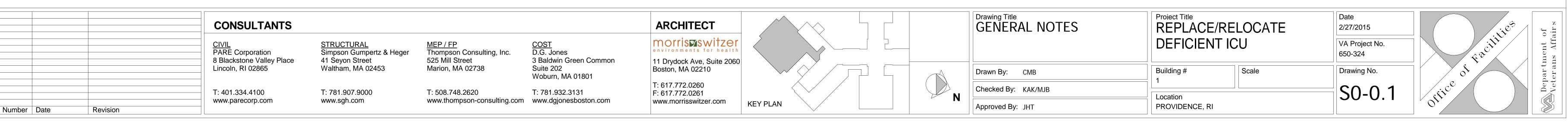
2/27/2015 VA Project No. 650-324 Drawing No. S0-0.0

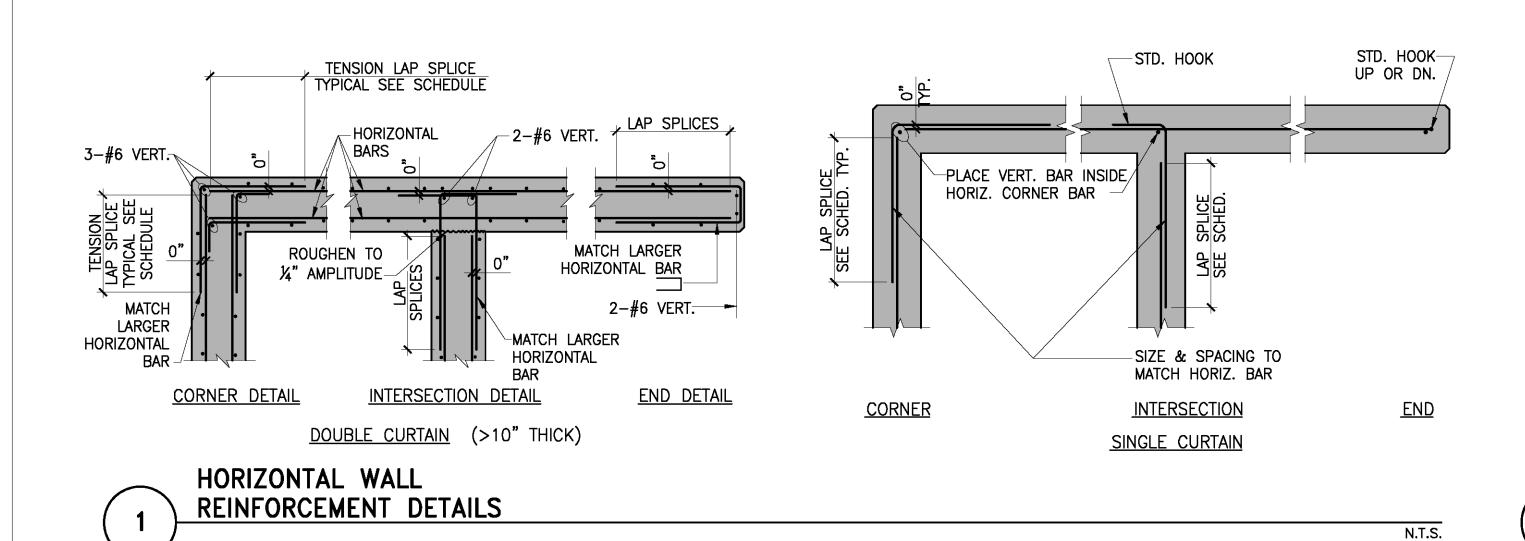
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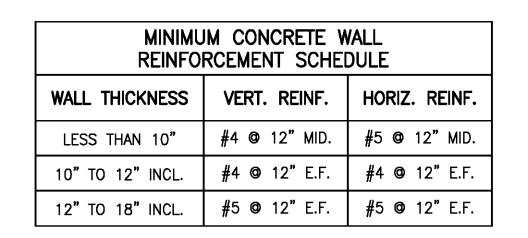


STRUCTURAL GENERAL NOTES **ABBREVIATIONS:** PART 1 — GENERAL REQUIREMENTS AND DESIGN CRITERIA 2.5 BACKFILL UNDER SLAB ON GRADE WORD or PHRASE WORD or PHRASE A. Backfill where required below slabs with approved granular soil placed in 8 to 10 in. layers and compacted to PART 4 - STRUCTURAL STEEL <u>ABBREVIATION</u> 95% density at optimum moisture content as defined by ASTM D-1557. Method Allowable Stress Design 1.1 GENERAL Kip (1000 pounds) A. Refer to project specifications for detailed requirements for material and workmanship. B. Use a polyethylene vapor barrier between the compacted structural fill and the slab-on-arade. 4.1 STRUCTURAL SHAPES Alternate Kips/Square Foot Above Finished Floor C. Do not leave bearing surface exposed to wet weather. B. Unless otherwise noted, details, sections, and notes contained in the structural contract documents shall be A. Wide Flange Shapes ASTM A992 (Fy = 50 ksi) ACI American Concrete Institute considered typical for all similar conditions even if not explicitly referenced. B. Hollow Structural Sections ASTM A500, Gr. B (Fy = 46 ksi) or ASTM 1085 (Fy=50ksi) LW Lightweight 2.6 FOUNDATION PLACEMENT & PROTECTION American Institute of Steel C. Deficient work and/or work not in conformance with the contract documents shall be repaired at the LWC Lightweight Concrete C. Angles ASTM A36, UON (Fy = 36 ksi) Construction A. Do not place foundation concrete in water or on frozen ground. LRFD Load & Resistance Factor contractor's expense. The contractor shall compensate the client for services arising from deficient work, D. Channels ASTM A36, UON (Fy = 36 ksi) American Iron and Steel B. Protect all soil bearing surfaces from freezing before and after foundation construction. AISI review of modifications/contractor substitution, or expediting of submittals. E. Plate ASTM A36, UON (Fy = 36 ksi) C. Protect in-place foundations and slabs from frost penetration until the project is complete. Long Leg Horizontal D. Cost of investigation and/or redesign incurred by the SER due to contractor errors will be at the contractor's ASTM A53, Type E, Grade B or ASTM A501 (Fy=42ksi) F. Pipe American Society for Testing D. Do not use salt or chloride compounds to de-ice the site. LLV Long Leg Vertical and Materials Low Point E. The contractor shall submit a single dimensioned and coordinated drawing for each level showing the locations 4.2 BOLTED CONNECTIONS 2.7 BACKFILL AGAINST WALLS AWS American Welding Society of all sleeves and openings required by all trades prior to initiating any work. A. ASTM A325/F1852 and A490/F2280 Do not backfill against walls until wall concrete is at full design strenath. Arch. or Arch't Architect Manufacturer Loads imposed on the base building structure and temporary conditions intended to accommodate construction B. Backfill with approved material placed in 8 in. layers and compacted to 95% density at optimum moisture Maximum means and methods are not explicitly considered in this design. The contractor shall advise the SER regarding 4.3 ANCHOR RODS content as defined by ASTM D-698, Method D. Mech. Mechanical construction loads and temporary conditions imposed on the building structure and shall compensate the SER A. ASTM F1554 Grade 55 rods (UON) with Supplementary Requirement S1 (weldability) Balance MEP+T Mechanical, Electrical, for reviewing these conditions. 2.8 DEWATERING SYSTEM Beam Plumbing and Technology 4.4 WELDING ELECTRODES A. Use temporary dewatering system to remove excess water from the excavation as required. B or Bot. Bottom Minimum 1.2 ELEVATIONS & DIMENSIONS A. Conform to AWS Specifications for electrodes based on welding process and the type and grade of steel. Bottom Each Way A. All dimensions, elevations and conditions shall be verified in the field by the contractors and any discrepancies B. E70XX electrodes (MIN.) for fillet welds 2.9 UTILITIES AND OTHER UNDERGROUND STRUCTURES Not In Contract shall be brought to the attention of the SER for clarification before proceeding with the affected part of the C. See project specifications for additional requirements for beam-column and column splice welds in moment CEP A. Foundations (footings and mat foundations) shall bear below a theoretical reference line drawn upward and Central Energy Plant Normal Weight Concrete work. Dimensions and elevations noted in the contract documents as (\pm) and all field conditions shall be outward on a 1V:2H slope from the bottom of any adjacent utilities or other underground structures. frames. Cast-in-Place NTS Not to Scale verified in the field (VIF) by the contractors prior to the submissions of shop drawings. Upon receipt of shop Center of Gravity No. or # Number drawings, the SER has the right to assume that all field dimensions, elevations and conditions have been 4.5 FABRICATION PART 3 - CONCRETE WORK Centered verified by the contractors and that the shop drawings accurately reflect such verifications unless stated A. Shop fabricate to greatest extent possible by welding, including beam stiffeners, column caps and bases, holes 0.C. Centerline On Center otherwise on the shop drawings. 3.1 CONCRETE MIX PROPERTIES Control Joint Outer Diameter B. Submit complete shop drawings from field dimensions for the Architect's approval of all structural steel prior to Complete Joint Penetration Weld 28-Day Design Maximum Opna Opening 1.3 BUILDING CODES AND REFERENCED STANDARDS Compressive Entrained Air fabrication. Density of A. 2012 International Building Code with Rhode Island Amendments (2013). Strength, f'c, (psi) Concrete (pcf) Max w/c Ratio Content(%) A. Element Column Pl. or Plate B. ASCE/SEI 7-10, Minimum Design Loads for Buildings and Other Structures Including Supplement No. 1. 4.6 ERECTION 1. Spread Footings, 4,000 psi $145 \pm 3\%$ 0.45 6% ± 1% Concrete Pounds/Linear Foot A. Provide anchor rods, steel wedges, threaded screws, or shims to support and plumb all columns. and Strip Foundation Concrete Masonry Unit Pounds/Square Foot 1.4 DESIGN LOADS B. Grout solid under base plates immediately after columns are plumb 2. Foundation Walls and Piers 4,000 psi $145 \pm 3\%$ 0.45 6% ± 1% Concrete Reinforcing Steel Institute Pounds/Sauare Inch A. Building Classification, Occupancy Risk Category IV. C. Provide bearing plates and wall anchors or anchor rods for all beams resting on concrete and all other 0.45 $145 \pm 3\%$ 0% (as mixed) 3. Interior Slabs-on-Grade 4.000 psi B. Dead Loads: Pre-cast necessary connecting hardware. 4. Exterior Slabs-on-Grade 5.000 psi $145 \pm 3\%$ 0.40 6% ± 1% Construction All permanent stationary construction including mechanical equipment and their weights where noted on the D. Set anchor rods using template and protect anchors prior to erection. Const. Jt. Construction Joint 5. Concrete on Metal Deck 4.000 psi $145 \pm 3\%$ 0.45 < 3% structural drawings. E. Do not field cut or field modify any structural steel without prior written approval by Architect for each specific 0.45 Continuous 6. Slab Depression Infill/Topping 4,000 psi $145 \pm 3\%$ < 3% Reinforce or Reinforcement C. Floor Live loads (uniform/concentrated): Curtain Wall ASTM C150, Type II. B. Portland Cement: accordance with Section 1607.9 of the Building Code. Remainder Where appropriate, these loads have been reduced i Admixtures: See project specifications for permissible admixtures. 100 psf / 1,000 lbs 4.7 PAINT 1. Corridors, at or below ground floor DBA Dowel Bar Anchor D. Fly Ash: See project specifications for permissible cementitious substitutes. Roof Drain 2. Stairs and Exits (not reducible) 100 psf A. Shop prime all steel not encased in concrete, not fireproofed, or not galvanized. Depression B. See Architectural Drawings and Specifications for finish coat requirements. 3. Typical Floor Diameter 3.2 BASE PLATE GROUT: See Architectural Drawings D. Live Load Reduction: Where appropriate, the live loads above have been reduced in accordance with Section Dimension A. 8,000 psi 28-day compressive strength, with fluid consistency. 4.8 HOT-DIP GALVANIZING 1607.9 of the Building Code. Dowels Shear Connector or Slip A. All steel, including but not limited to structural members, connection materials, brick relieving angles, loose E. Roof Snow/Rain Load Parameters Dwg Drawing 3.3 STEEL REINFORCEMENT Critical Bolt lintels, and misc. metals, that is exposed to the exterior elements (weather) shall be hot—dip galvanized. Where appropriate, drifting snow loads have been considered in accordance with Section 7.7 of ASCE/SEI 7-10. A. ASTM A615 Grade 60, deformed. Slab Control Joint B. Galvanized members that are to be field welded shall have weld-affected areas masked prior to galvanizing 1. Ground Snow Load, P. B. ASTM A706, deformed where rebar is indicated to be welded Existing 30 psf Structural Engineer of C. All field welds, or areas where hot—dip galvanizing is damaged, shall be touched—up with a zinc—rich paint . ASTM A995 Stainless Steel Type 304 Grade 60, deformed. 2. Flat Roof Snow Load, ("cold galvanizing") after steel is completely installed, including exposed end of tension controlled bolts. D. Do not tack or spot—weld crossing bars. 30 psf Each End a. Roof over occupied space, heated Headed Shear Stud Each Face 30 psf ASTM A497 welded wire reinforcement. (Use flat sheets only) b. Canopy/overhang roofs Sht Sheet 4.9 FRAMING 1.0 F. Headed deformed bars may be used in lieu of hooks in congested areas. Place per ACI 318-11. Each Side 3. Snow Exposure Factor, Ce Sim. Similar A. Beams are equally spaced, UON. Each Way 4. Snow Load Importance Factor, k 1.2 Steel Joist Institute B. Cantilever beams are same size as back span. UON. Elevation 3.4 REINFORCEMENT AT OPENINGS 5. Thermal Factor, C Spaces C. Bolt patterns shown on details illustrate the concept of the connection and do not necessarily show the 1.2 Elevator a. For Unheated Canopy Roofs A. UON, provide 2 - #6 at each side of all openings in walls and slabs, and extend 2 ft-6 in. beyond the Slab-on-Grade actually number and arrangement of the bolts in the connection, unless specifically detailed. Edge of Slab 1.0 b. For Other Roofs opening or as detailed, except vertical bars at sides of openings in walls, which are to extend from floor to Sauare Engineer of Record F. Wind Load Parameters Stainless Steel 4.10 STANDARD SPECIFICATIONS Embed Plate Basic Wind Speed (Nominal 3 second aust). V 144 mph (Risk Category IV) B. Upon Approval by SER, bars may be moved aside at openings or sleeves, but do not cut or omit. Standard A. AISC 360-10 Specification for Structural Steel Buildings 2. Wind Exposure B. AISC 341-10 Seismic Provisions for Structural Steel Buildings, Including Supplement No.1 Exp. Bolt **Expansion Bolt** 3. Internal Pressure Coefficient 3.5 SPLICING OF REINFORCEMENT: ±0.18 Steel Deck Institute C. AWS D1.1 Structural Welding Code — Steel A. Refer to table in typical details Expansion Joint 4. Design Wind Pressure: Components and Cladding Stiffener Exterior a. To be determined in accordance with the governing building code as required B. Tie bars together at lap. Structural 4.11 STRUCTURAL TESTS AND INSPECTIONS G. Seismic Load Parameters Sump Pit A. Structural tests and inspections are required for this project. Refer to program of structural tests and special 3.6 MINIMUM REINFORCEMENT inspections for requirements. 0.176 A. Reinforce all walls with at least #4 @ 12 in. each way each face and 2 - #6 each edge. Finish Spectral Response Acceleration, S Fin. **Symmetrical** Spectral Response Acceleration. S. 0.062 Fin. Fl Finish Floor B. In slabs, provide at least 0.0018 times the area of concrete in each direction. PART 5 - STEEL DECK AND SHEAR STUDS 4. Site Class T&B Top & Bottom 0.141 Floor Drain Spectral Response Coefficient, Sps. 3.7 REINFORCEMENT SHOP DRAWINGS Top of Concrete 5.1 STEEL DECK 0.070 Footing Spectral Response Coefficient, Sp. A. Submit for approval, complete bending and placing details of all reinforcement, indicating position of splices. Top of Steel A. Provide steel deck made from adjugatized steel with minimum yield strength of 33 ksi. UON. Seismic Design Category Foundation B. Include accessory drawings. Top of Wall B. See Drawings and Specifications for gauge and profile. 8. Long-Period Transition Period, 1 Fire Protection C. Include locations of all sleeves cast into concrete to accommodate mechanical trades. Contractor shall Typical C. Provide sheet metal pour stops with thickness based on SDI criteria (SDI Publication #31); 14 gauge min. 9. Base Elevation At Grade Footing Steps coordinate all sleeve locations. 10. Seismic Force Resisting System: D. Include all conduit, piping, ductwork, etc. to be embedded within the structural concrete for approval by the UON Unless Otherwise Noted D. All Steel Deck and supporting members are sized and spaced assuming at least a two span condition for the a. Ordinary Steel Moment Frames not specifically detailed for seismic resistance Galvanized SER. Contractor shall coordinate all embed locations. metal deck. The steel deck supplier, installer and general contractor shall coordinate installation and shoring 1) Resource modification factor R = 3.0Gauge V or Vert Vertical requirements for single span deck. 3.8 CONCRETE CLEAR COVER Deflection amplification factor Cd = 3.0Vertical Each Face 11. Seismic Response Coefficient. Cs A. Concrete placed against earth Grade Beam 0.070 (Eq. 12.8-2) Vertical Inside Face 5.2 HEADED STUDS B. Slabs-on-grade top 1 in. 87 kips 12. Base Shear, V Verify in Field A. Provide headed type studs which conform to ASTM A108 Grade 1010 or 1020 cold finished carbon steel. High Point C. Formed concrete exposed to earth, water, or weather 13. Analysis Procedure VOF Vertical Outside Face Hollow Structural Section D. Interior faces of walls 1 in. a. Equivalent Lateral Force B. Provide $\frac{3}{4}$ in. diameter by 6 in. long studs, UON. H or Horiz. E. Exterior faces of walls 2 in. H. Seismic Load Parameters (Non-Structural Components) C. See the drawings for number and locations of studs Welded Wire Reinforcement Horizontal Each Face F. Column or Piers (primary reinforcement) 2 in. Component Importance Factor, le D. Space studs uniformly along length of beam, UON. Horizontal Inside Face Varies by component Component Amplification Factor, ap Working Point E. Provide a minimum of 1 in. from the edge of any stud and the face of concrete, a metal deck rib or similar discontinuity. HOF Horizontal Outside Face 3.9 POST-INSTALLED ANCHORS Component Response Modification Factor, Rp Varies by component Waterproofing (Coord. with F. Where composite steel beams on plan are missing a shear stud designation, provide the following minimum number of shear A. Expansion Anchors: Hilti Kwik Bolt 3, 5%" min. 4. Horizontal Design Force, Fp (for major equipment above grade) Varies by component Architectural Dwgs) Install per Hilti installation recommendations. 1.5 BEAM DEFLECTION CRITERIA 1. Beams designated as part of lateral force—resisting system: 1 stud per foot Inclusive or Including Provide standard depth of embedment as listed by Hilti, UON. A. Composite Steel Beams 2. All other beams: 1 stud per 2 feet Information 3. Provide Stainless Steel anchors and hardware in all exterior applications. 1. Post-Composite Live Load Deflection span/360 2. Post-Composite Superimposed Deflection, Typical Conditions span/240 B. Adhesive Anchors: Hilti HIT-HY 200 Injection Adhesive Anchors 5.3 STANDARD SPECIFICATIONS 3. Post-Composite Superimposed Deflection, Supporting Brick Veneer lesser of span/600 or 0.375 in. 1. Install per Hilti installation recommendations. A. AISC Specifications per PART 16, Chapter I 4. Post-Composite Superimposed Deflection, Supporting Curtain Wall lesser of span/360 or 0.5 in. 2. Provide standard depth of embedment as listed by Hilti, UON. B. AISI Specification for the Design of Cold-Formed Steel Structural Members 5. Net Total Deflection 0.75 in. 3. Do not use in an overhead application. C. SDI Code of Recommended Practice and Specifications for Composite Steel Floor Deck B. Non-Composite Steel Beams 4. Provide Stainless Steel anchors and hardware in all exterior applications. 1. Live Load Deflection span/360 D. AWS Structural Welding Code — Steel and Structural Steel Welding Code — Sheet Steel 2. Net Total Deflection, Typical Conditions lesser of span/240 or 1.25 in. 3.10 EXISTING SURFACE TREATMENT 3. Net Total Deflection, Supporting Brick Veneer lesser of span/600 or 0.25 in. 5.4 STRUCTURAL TESTS AND INSPECTIONS A. Roughen all existing concrete surfaces common with new concrete to amplitude of 1/4 inch. lesser of span/360 or 0.75 in. 4. Net Total Deflection, Supporting Curtain Wall B. Existing concrete shall also be considered concrete on this job at construction joints or where a secondary A. Structural tests and inspections are required for this project. Refer to program of structural tests and special inspections for requirements. PART 2 - FOUNDATIONS 3.11 HOUSEKEEPING PADS AND CURBS A. Pads and curbs may be shown on plan in certain instances for reference only. See Architectural and 2.1 REFERENCE GEOTECHNICAL REPORT A. Foundation design is in accordance with the recommendations provided by the following: Mechanical Drawings and Specifications and coordinate with equipment manufacturer's requirements and location. 1. GZA GeoEnvironmental. Inc.: Geotechnical Report New Emergency Department and Intensive Care Unit, Veterans Affairs B. Provide the same concrete as base slab, UON. Medical Center, Providence, Rhode Island, dated March 2011 SYMBOL LEGEND 2. PARE Corporation: Geotechnical Report Specialty Clinic Addition and Upgrade Electrical System, Veterans Affairs Medical 3.12 STANDARD SPECIFICATIONS AND REFERENCE STANDARDS Center, Providence, Rhode Island, dated July 2009. A. ACI 301 Specification for Structural Concrete B. Copies of the geotechnical report are available from the project Architect. The Contractor shall be responsible B. CRSI Manual of Standard Practice BEAM TO COLUMN MOMENT CONNECTION (PART OF LATERAL FORCE—RESISTING SYSTEM) NEW CAST IN PLACE CONCRETE for reading, understanding, and implementing the recommendations outlined in the Geotechnical Report. Where C. Follow the latest recommendations and specifications of the American Concrete Institute: recommendations in the report vary from information contained in these drawings and the project specifications, 1. ACI 302 Concrete Floor and Slab Construction the more stringent recommendation shall govern. 2. ACI 304 Measuring, Mixing, Transporting and Placing Concrete SLAB OR ROOF DECK OPENING BEAM TO COLUMN CANTILEVER MOMENT CONNECTION 3. ACI 305 Hot Weather Concreting Cold Weather Concreting 4. ACI 306 A. Spread Footings: designed for a maximum allowable bearing pressure of 4.0 ksf and are to bear on compacted 5. ACI 315 Detailing for Reinforcing COMPACTED FILL OR UNDISTURBED SOIL structural fill or glacial till. Over excavate to a minimum of 12 in, below footings, and backfill with compacted 6. ACI 318 General Design of Items Not Otherwise Specified BEAM TO BEAM CANTILEVER MOMENT CONNECTION structural fill. 7. ACI 347 GRANULAR FILL 2.3 EXCAVATION 3.13 CONSTRUCTION JOINTS STANDARD BEAM CONNECTION A. All foundation excavation to be inspected by the Special Inspector A. Refer to Typical Details. B. The elevations shown on the drawings are anticipated and actual elevations are to be established in the field INDICATES COLUMN STARTS AT THIS LEVEL by the Geotechnical Engineer. BEAM TO CONC. FOUNDATION WALL CONNECTION C. In no case shall the bottom of foundation be located less than 3'-4" below the lowest adjacent surface A. All curing compounds must be approved by the SER and Architect. See specifications for requirements. INDICATES COLUMN STOPS AT THIS LEVEL exposed to freezing. B. Curing compounds shall not be used on flatwork. INDICATES COLUMN CONTINUES DIRECTION OF DECK SPAN 2.4 SUBGRADE PREPARATION 3.15 CONCRETE INFILL AT SLABS DEPRESSIONS A. Follow recommendations of the geotechnical report included in the project manual. A. Use $\frac{1}{3}$ " maximum aggregate size to infill interior slab depressions. INDICATES BOTTOM OF FOOTING ELEVATION B. Proof roll and compact existing soils to 95% density at optimum moisture content as defined by ASTM D-1557. FACTORED AXIAL FORCE ON MEMBER (KIPS) JOHN H. THOMSEN V = FACTORED SHEAR FORCE ON MEMBER (KIPS) 3.16 EDGE OF SLAB BEAM PENETRATION C. Place all spread and strip footings on 12 in. compacted structural fill. A. Refer to architectural edge of slab plans for extents and locations of slab edges, house keeping pads, and No. REGISTERED 2-2: - 2015 FACTORED MOMENT ON MEMBER (KIP-FEET) D. Do not leave bearing surface of subgrade exposed to wet weather. curbs not otherwise shown on drawings. **CENTERLINE** 3.17 STRUCTURAL TESTING AND INSPECTIONS NUMBER OF HEADED SHEAR STUDS A. Absolutely no concrete is to be placed prior to rebar being inspected and approved. AMOUNT UPWARD OF BEAM CAMBER C = B. Refer to program of structural tests and special inspections for additional requirements. PROFESSIONAL ENGINEER 16

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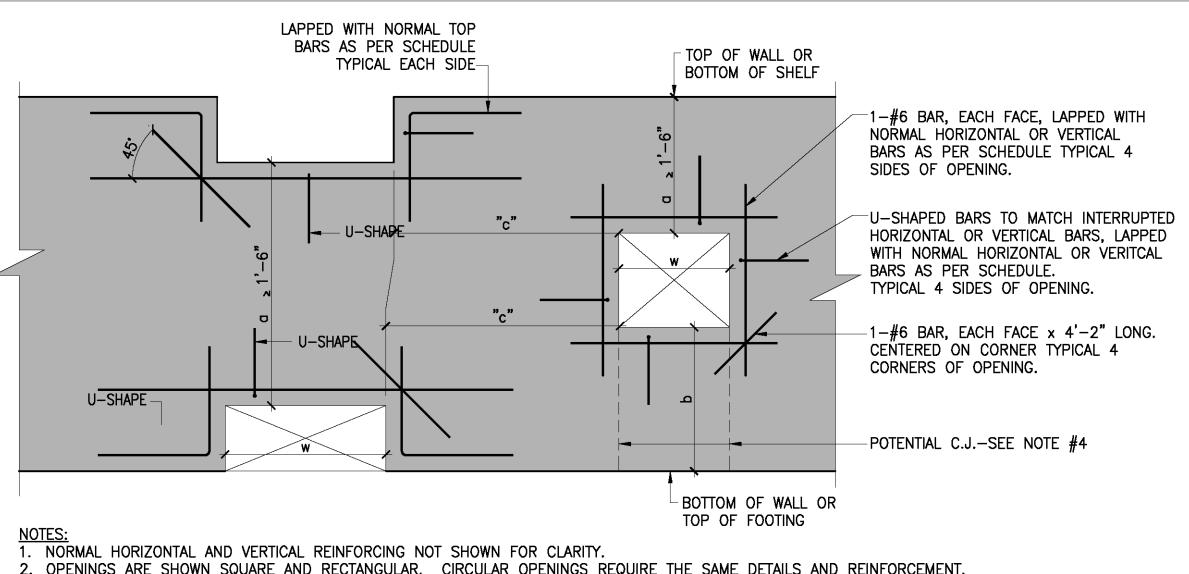






NOTE:
PROVIDE THE ABOVE MINIMUM REINFORCEMENT IN ALL CONCRETE WALLS WHERE REINFORCEMENT IS NOT INDICATED ON THE DRAWINGS

MINIMUM WALL REINFORCEMENT SCHEDULE



2. OPENINGS ARE SHOWN SQUARE AND RECTANGULAR. CIRCULAR OPENINGS REQUIRE THE SAME DETAILS AND REINFORCEMENT. 3. WHEN DIMENSION "a" IS LESS THAN 0.5 w, REPLACE U-SHAPED BARS WITH CLOSED STIRRUPS. SPACE CLOSED STIRRUPS TO MATCH

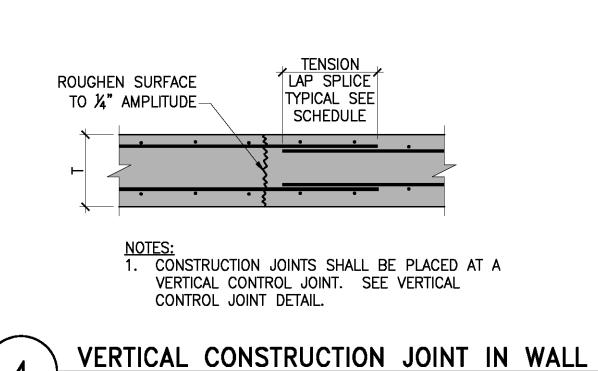
VERTICAL WALL REINFORCING BUT NOT GREATER THAN "a"/2. 4. WHERE DIMENSION "b" IS SMALL AND PLACING/FINISHING OF CONCRETE BELOW OPENING IS DIFFICULT, THE GENERAL CONTRACTOR MAY ELECT TO ADD VERTICAL CONSTRUCTION JOINTS THAT ALIGN WITH THE EDGES OF THE OPENINGS, PROVIDED THAT ALL REINFORCEMENT IS

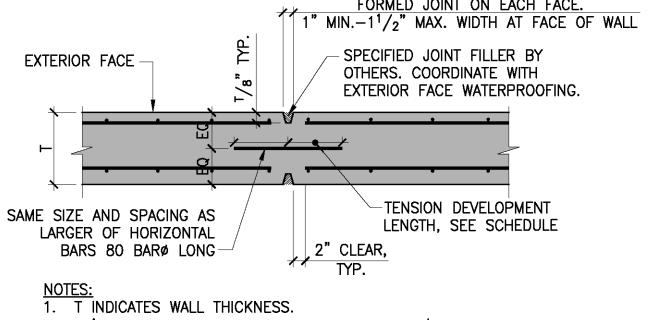
CONTINUOUS THROUGH THE JOINTS. 5. WHEN DIMENSION "c" IS LESS THAN 6'-0", THE ENGINEER MAY REQUIRE SPECIAL REINFORCEMENT AROUND THE OPENINGS. SUBMIT THESE

OPENINGS TO THE ENGINEER FOR REVIEW. 6. REFER TO ALL THE CONTRACT DRAWINGS TO DETERMINE THE SIZES AND LOCATIONS OF ALL OPENINGS THROUGH THE CONCRETE WALLS.

7. HOLES IN SHEAR WALLS NOT SPECIFICALLY DETAILED IN THE DRAWINGS ARE PROHIBITED, UNLESS NOTED ON PLAN.

OPENING THROUGH CONCRETE WALL DETAIL

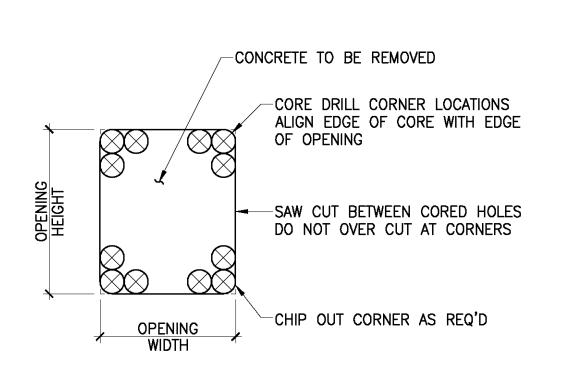




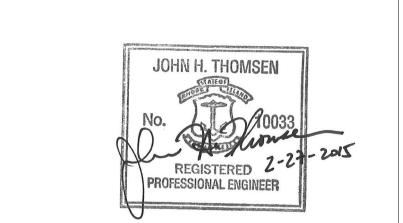
FORMED JOINT ON EACH FACE.

2. 60' MAXIMUM SPACING BETWEEN CONTROL AND/OR CONSTRUCTION JOINTS. 3. PROVIDE JOINTS WITHIN 5 FT. OF ALL CORNERS AND/OR INTERSECTIONS.





TYPICAL OPENING DETAIL THROUGH (E) CONCRETE WALL OR SLAB



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COST D.G. Jones 3 Baldwin Green Common Suite 202 Woburn, MA 01801 T: 781.932.3131 www.thompson-consulting.com www.dgjonesboston.com

ARCHITECT morris switzer
environments for health 11 Drydock Ave, Suite 2060 Boston, MA 02210 T: 617.772.0260 F: 617.772.0261 www.morrisswitzer.com **KEY PLAN**

	Drawing Title TYPICA DETAIL	
	Drawn By:	CMB
NI I	Checked By:	KAK/MJB
IN	Approved By:	JHT

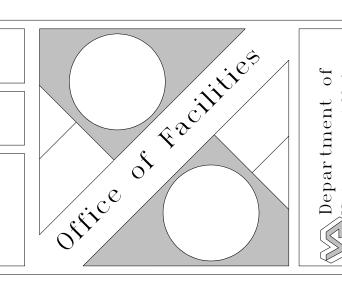
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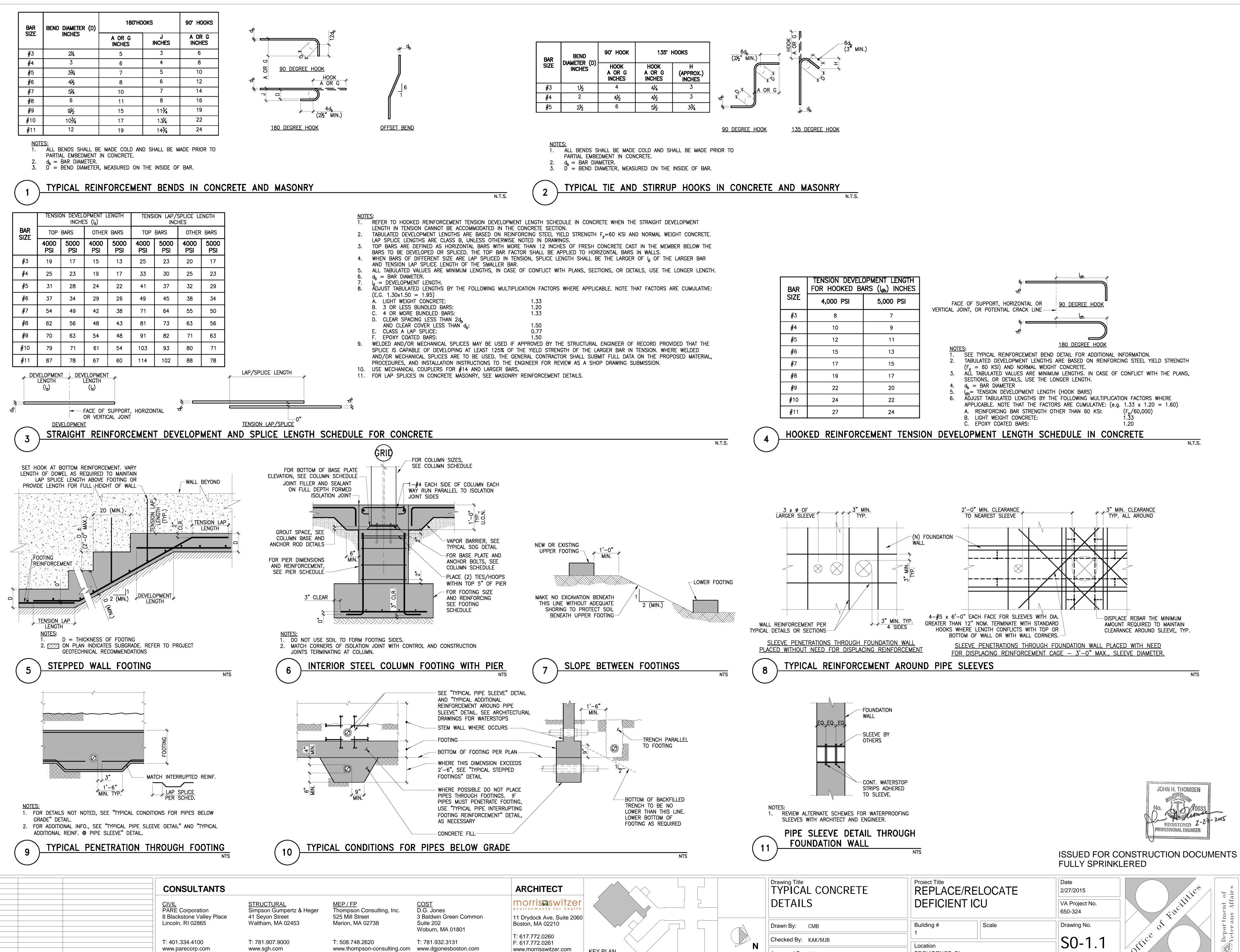
Project Title REPLACE/RELOCATE **DEFICIENT ICU** Scale Building #

Location

PROVIDENCE, RI

2/27/2015 VA Project No. 650-324 Drawing No.





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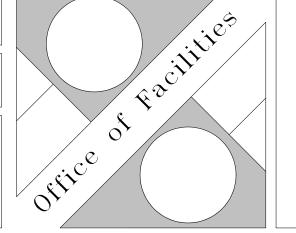
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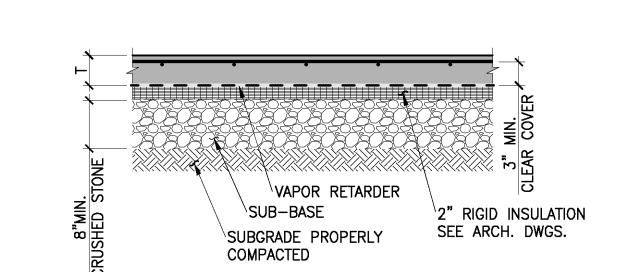
Number Date

KEY PLAN

Approved By: JHT

PROVIDENCE, RI



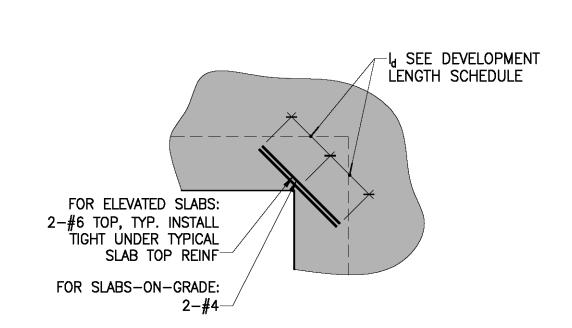


NOTES:

1. SEE PLANS FOR SLAB CONSTRUCTION.

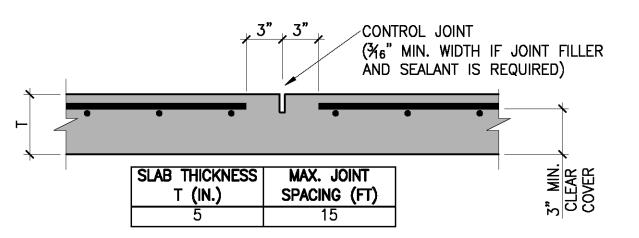
- 2. SEE PLANS, GENERAL NOTES AND/OR SPECIFICATIONS FOR VAPOR BARRIER. 3. SEE PLANS, GENERAL NOTES AND/OR SPECIFICATIONS FOR SUB-BASE
- AND/OR SUBGRADE REQUIREMENTS. 4. SEE GENERAL NOTES FOR ADDITIONAL REINFORCING BAR CONCRETE COVER REQUIREMENTS.

TYPICAL SLAB-ON-GRADE DETAIL



NOTE:
DETAIL APPLIES AT OPENINGS AND CORNERS OF SLAB DEPRESSIONS.

TYPICAL SLAB REINFORCEMENT AT RE-ENTRANT CORNER



1. SAW CUT 1" (MIN.) DEEP JOINT WITH EARLY ENTRY SAW AS SOON AS POSSIBLE AFTER FINISHING SLAB. (4 TO 8 HOURS MAX.)

- DETAIL REINFORCEMENT TO STOP AT JOINTS AS INDICATED. LOCATE CONTROL JOINTS ON A RECTANGULAR GRID AND SUBMIT LAYOUT OF CONTROL JOINTS FOR APPROVAL UNLESS JOINT LOCATIONS ARE OTHERWISE SPECIFIED. RE-ENTRANT CORNERS ARE DISCOURAGED. WHERE UNAVOIDABLE, PROVIDE CORNER BARS PER DETAIL. TYPICAL SLAB REINFORCEMENT AT RE-ENTRANT CORNER. ASPECT RATIO OF RECTANGLES SHALL NOT EXCEED 1.25.
- SEE PLANS, GENERAL NOTES AND/OR SPECIFICATIONS FOR JOINT FILLER AND REINFORCEMENT SHOWN GRAPHICALLY. REFER ELSEWHERE FOR SIZE AND SPACING. SEE GENERAL NOTES FOR ADDITIONAL REINFORCING BAR CLEAR COVER REQUIREMENTS.

-BOX-OUT AROUND COLUMN AND FILL WITH CONCRETE

AFTER COLUMN AND SLAB

 $\operatorname{\mathsf{-PREFORMED}}$ ISOLATION $_{f}$

-STEEL OR CONCRETE

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NOTES:

1. REMOVE TOP INCH OF PREFORMED ISOLATION JOINT FILLER AND SEAL JOINT AS REQUIRED FOR CONTROL JOINTS.

2. AT PINWHEEL PATTERN, CONTRACTOR HAS OPTION OF PLACING ISOLATION JOINT TIGHT TO COLUMN AND SAW CUT PINWHEEL PATTERN.

JOINT FILLER -

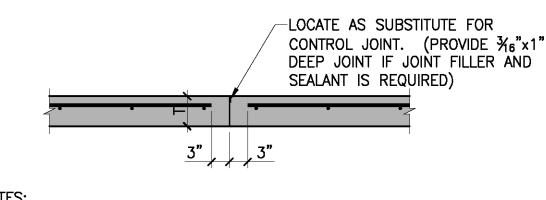
SLAB-ON-GRADE ISOLATION JOINT

DIAMOND PATTERN

DETAILS AT COLUMNS

CONSTRUCTION

CONTROL JOINT IN SLAB-ON-GRADE



1. DETAIL REINFORCEMENT TO STOP AT JOINT. 2. SUBMIT LOCATIONS OF CONSTRUCTION JOINTS FOR APPROVAL. 3. ALLOW A MINIMUM OF 72 HOURS BETWEEN ADJACENT POURS. 4. SEE SPECIFICATIONS FOR JOINT FILLER AND SEALANT REQUIREMENTS.

SLAB-ON-GRADE CONSTRUCTION JOINT

BOX-OUT AROUND COLUMN

AND FILL WITH CONCRETE

6" MIN. UNLESS

-STEEL OR CONCRETE

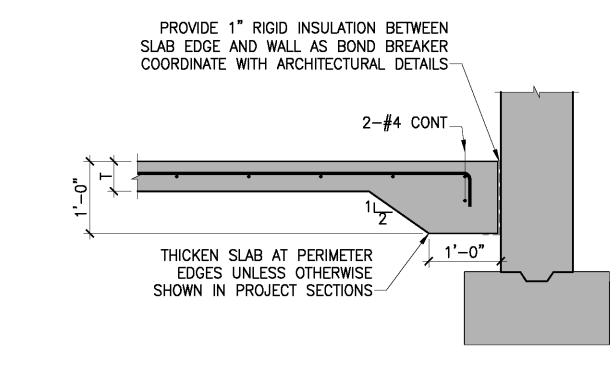
COLUMN

ROUND (SONOTUBE) PATTERN

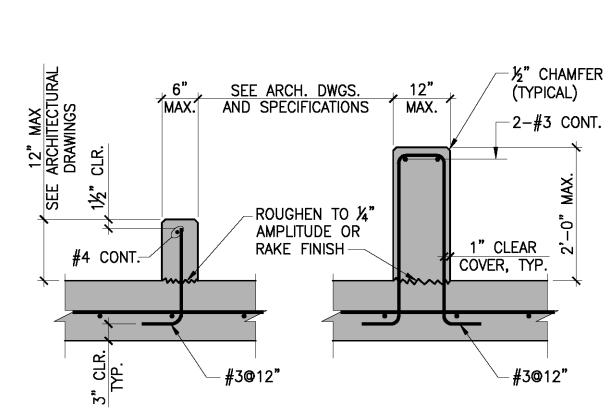
OTHERWISE SPECIFIED

CONSTRUCTION

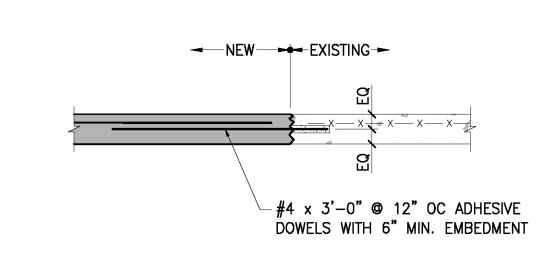
AFTER COLUMN AND SLAB



THICKENED SLAB-ON-GRADE AT PERIMETER



1. ADHESIVE DOWELS ARE AN ACCEPTABLE ALTERNATIVE. MATCH REINFORCING BAR SIZE AND SPACING SHOWN. PROVIDE A MINIMUM EMBEDMENT OF 21/2".



MIN. OF 2T OR 12"

HORIZONTAL CONSTRUCTION

ROUGHEN TO 1/4" AMPLITUDE

MIN. OF 2T, 12", AND

TENSION DEVELOPMENT LENGTH

2. SEE PLANS FOR SLAB CONSTRUCTION.

JOINT AT CONTRACTOR'S OPTION.

CONCRETE CURB AT SLAB-ON-GRADE
SIMILAR AT ELEVATED CONCRETE SLAB

NTS



/-#4 OFFSET BARS @ 15" OC

-#4 Z-BARS @ 15" OC. PROVIDE STANDARD

90' BEND W/ FULL DEVELOPEMENT LENGTH

DEPRESSION < 24"

T+2" < SLAB

-CONTINUOUS #4 BARS TOP,

BOTTOM AND 8" OC FOR BALANCE

TOP & BOTTOM

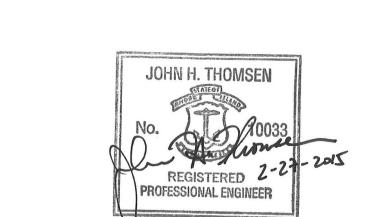
. [/////] ON DETAIL INDICATES SUBGRADE. REFER TO TYPICAL SLAB-ON-GRADE

DETAIL, GENERAL NOTES AND/OR SPECIFICATIONS.

DEPRESSIONS AT SLAB-ON-GRADE

★ SLAB DEPRESSION < T+2"

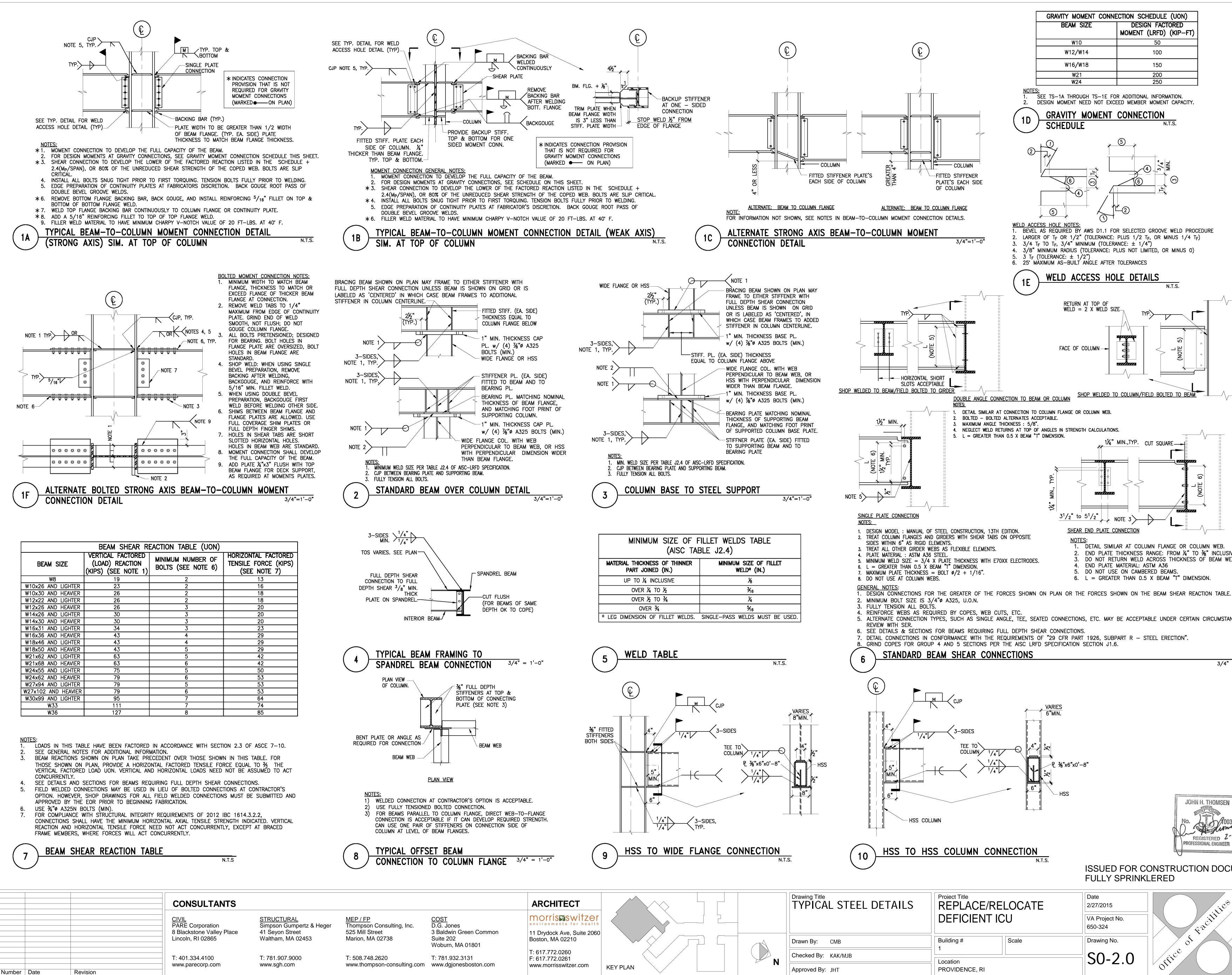
TENSION LAP LENGTH



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										TOLET OF KINKLE	-'
	CONSULTANTS				ARCHITECT			Drawing Title TYPICAL CONCRETE	Project Title REPLACE/RELOCATE	Date 2/27/2015	
	CIVIL PARE Corporation	STRUCTURAL Simpson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	<u>COST</u> D.G. Jones	morris switzer environments for health			DETAILS	DEFICIENT ICU	VA Project No. 650-324	
	8 Blackstone Valley Place Lincoln, RI 02865	41 Seyon Street Waltham, MA 02453	525 Mill Street Marion, MA 02738	3 Baldwin Green Common Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210			Drawn By: CMB	Building # Scale	Drawing No.	
	T: 401.334.4100 www.parecorp.com	T: 781.907.9000 www.sgh.com	T: 508.748.2620 www.thompson-consulting.com	T: 781.932.3131 www.dgjonesboston.com	T: 617.772.0260 F: 617.772.0261 www.morrisswitzer.com	KEY PLAN	N	Checked By: KAK/MJB	Location	S0-1.2	
ber Date Revision						TALL I LIVIN		Approved By: JHT	PROVIDENCE, RI		`

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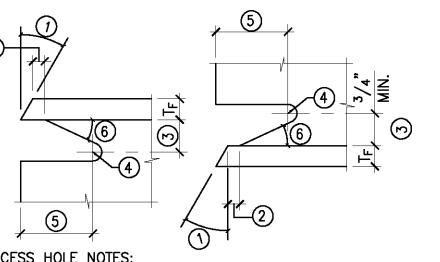
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GRAVITY MOMENT CONNECTION SCHEDULE (UON) BEAM SIZE DESIGN FACTORED MOMENT (LRFD) (KIP-FT) W10 W12/W14 100 W16/W18 150

SEE TS-1A THROUGH TS-1E FOR ADDITIONAL INFORMATION. DESIGN MOMENT NEED NOT EXCEED MEMBER MOMENT CAPACITY.

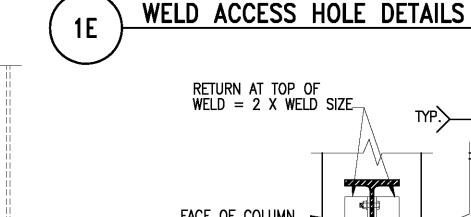
GRAVITY MOMENT CONNECTION SCHEDULE



WELD ACCESS HOLE NOTES: . BEVEL AS REQUIRED BY AWS D1.1 FOR SELECTED GROOVE WELD PROCEDURE LARGER OF T_F OR 1/2" (TOLERANCE: PLUS 1/2 T_F, OR MINUS 1/4 T_F)

3. 3/4 T_F TO T_F, 3/4" MINIMUM (TOLERANCE: $\pm 1/4$ ") 4. 3/8" MINIMUM RADIUS (TOLERANCE: PLUS NOT LIMITED, OR MINUS 0) 5. 3 T_F (TOLERANCE: $\pm 1/2$ ")

6. 25" MAXIMUM AS-BUILT ANGLE AFTER TOLERANCES



FACE OF COLUMN --SHOP WELDED TO COLUMN/FIELD BOLTED TO BEAM DOUBLE ANGLE CONNECTION TO BEAM OR COLUMN DETAIL SIMILAR AT CONNECTION TO COLUMN FLANGE OR COLUMN WEB.

5. MINIMUM WELD SIZE = 3/4 X PLATE THICKNESS WITH E70XX ELECTRODES

1. DETAIL SIMILAR AT COLUMN FLANGE OR COLUMN WEB. 2. END PLATE THICKNESS RANGE: FROM ¼" TO ¾" INCLUSIVE DO NOT RETURN WELD ACROSS THICKNESS OF BEAM WEB. 4. END PLATE MATERIAL: ASTM A36

1¼" MIN.,TYP. CUT SQUARE—

DO NOT USE ON CAMBERED BEAMS. 6. L = GREATER THAN 0.5 X BEAM "T" DIMENSION.

 $3^{1}/2$ " to $5^{1}/2$ " NOTE 3

SHEAR END PLATE CONNECTION

4. REINFORCE WEBS AS REQUIRED BY COPES, WEB CUTS, ETC. 5. ALTERNATE CONNECTION TYPES, SUCH AS SINGLE ANGLE, TEE, SEATED CONNECTIONS, ETC. MAY BE ACCEPTABLE UNDER CERTAIN CIRCUMSTANCES.

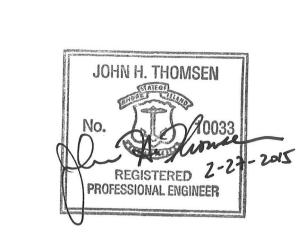
6. SEE DETAILS & SECTIONS FOR BEAMS REQUIRING FULL DEPTH SHEAR CONNECTIONS.

7. DETAIL CONNECTIONS IN CONFORMANCE WITH THE REQUIREMENTS OF "29 CFR PART 1926, SUBPART R — STEEL ERECTION".

. GRIND COPES FOR GROUP 4 AND 5 SECTIONS PER THE AISC LRFD SPECIFICATION SECTION J1.6.

VARIES 6"MIN.

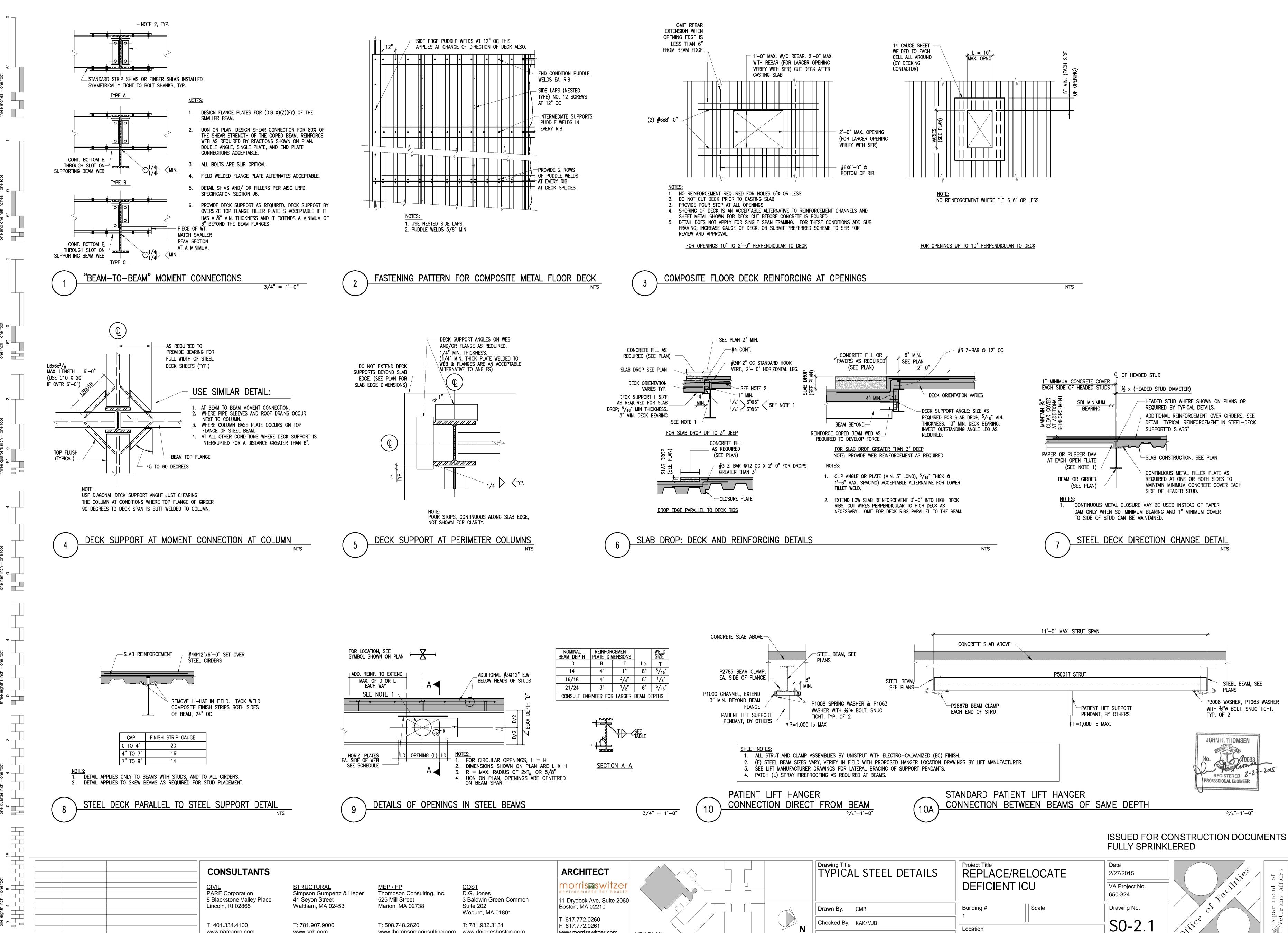
HSS TO HSS COLUMN CONNECTION



3/4" = 1'-0"

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REPLACE/RELOCATE 2/27/2015 VA Project No. 650-324 Scale Drawing No. S0-2.0



Location

Approved By: JHT

PROVIDENCE, RI

Number Date

Revision

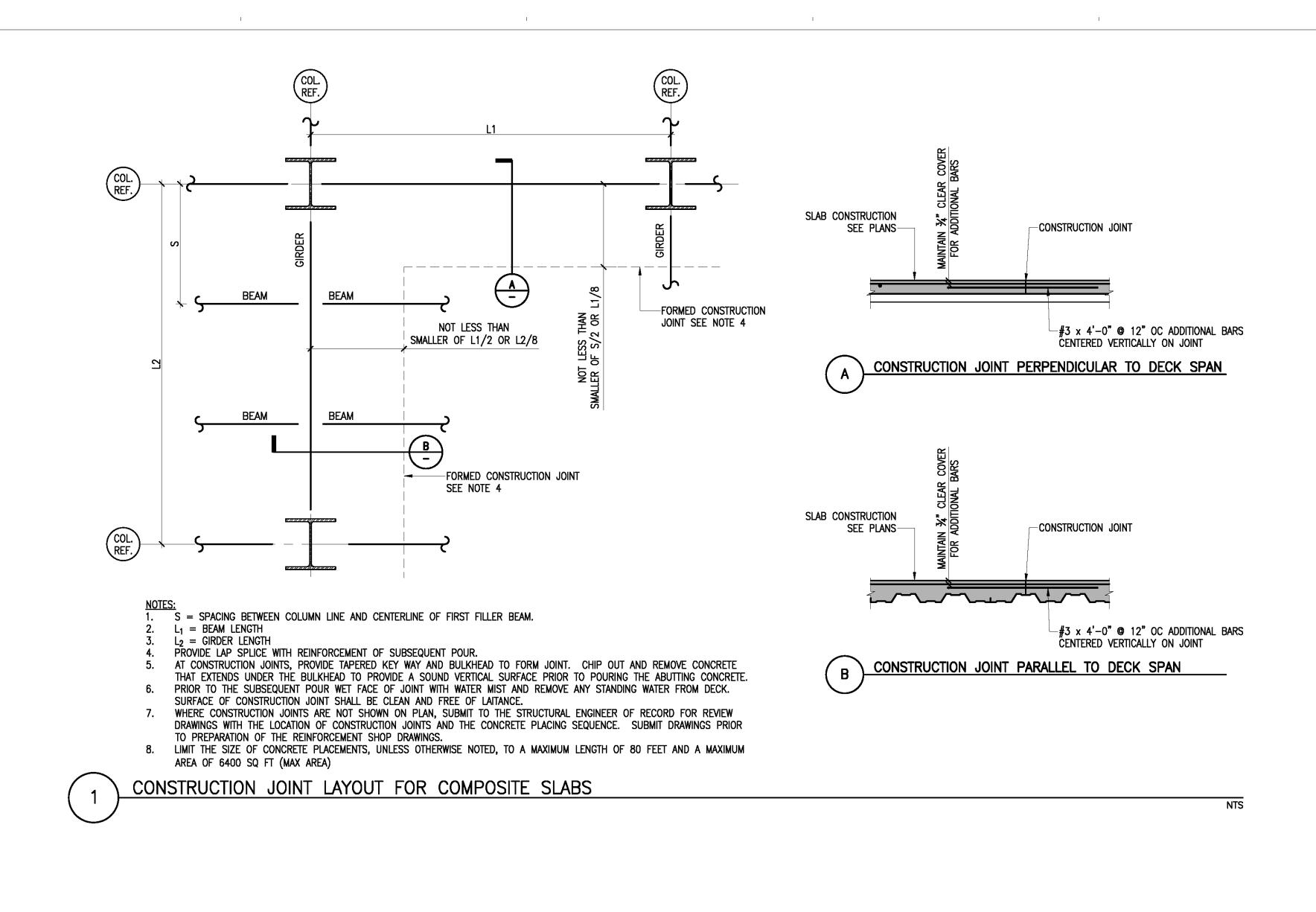
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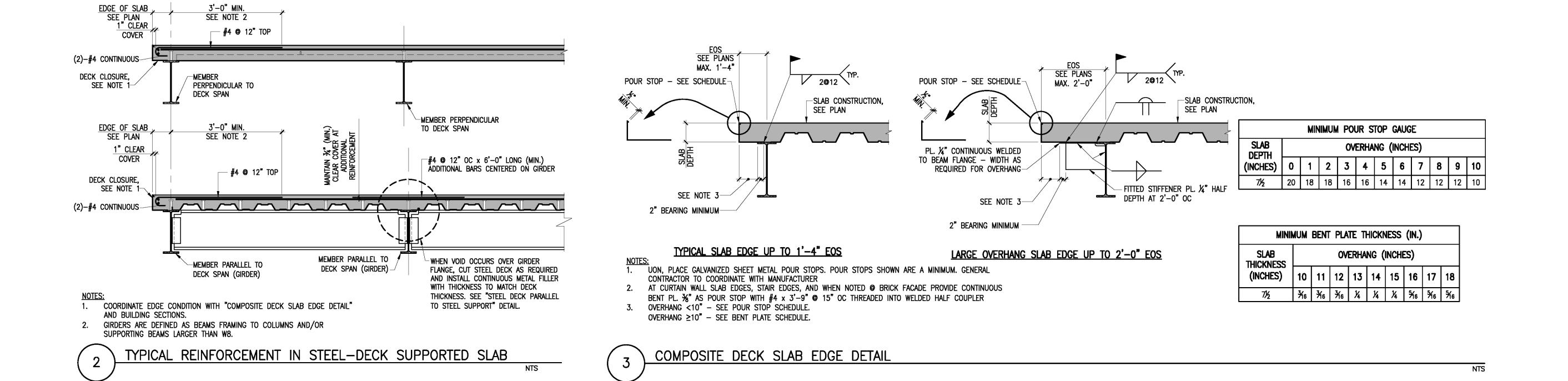
www.sgh.com

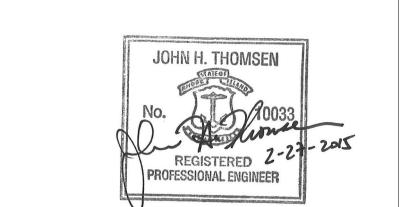
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KEY PLAN

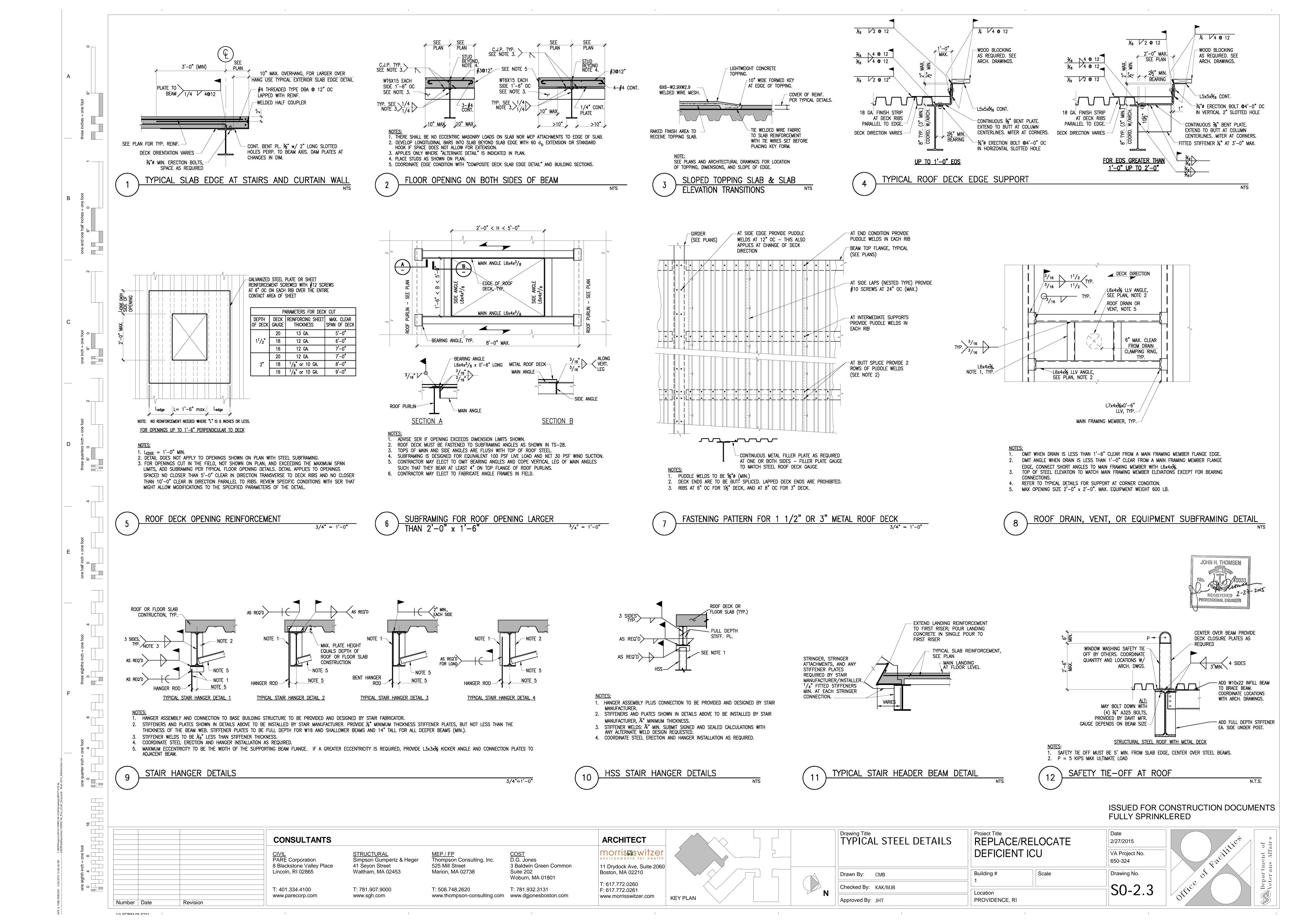


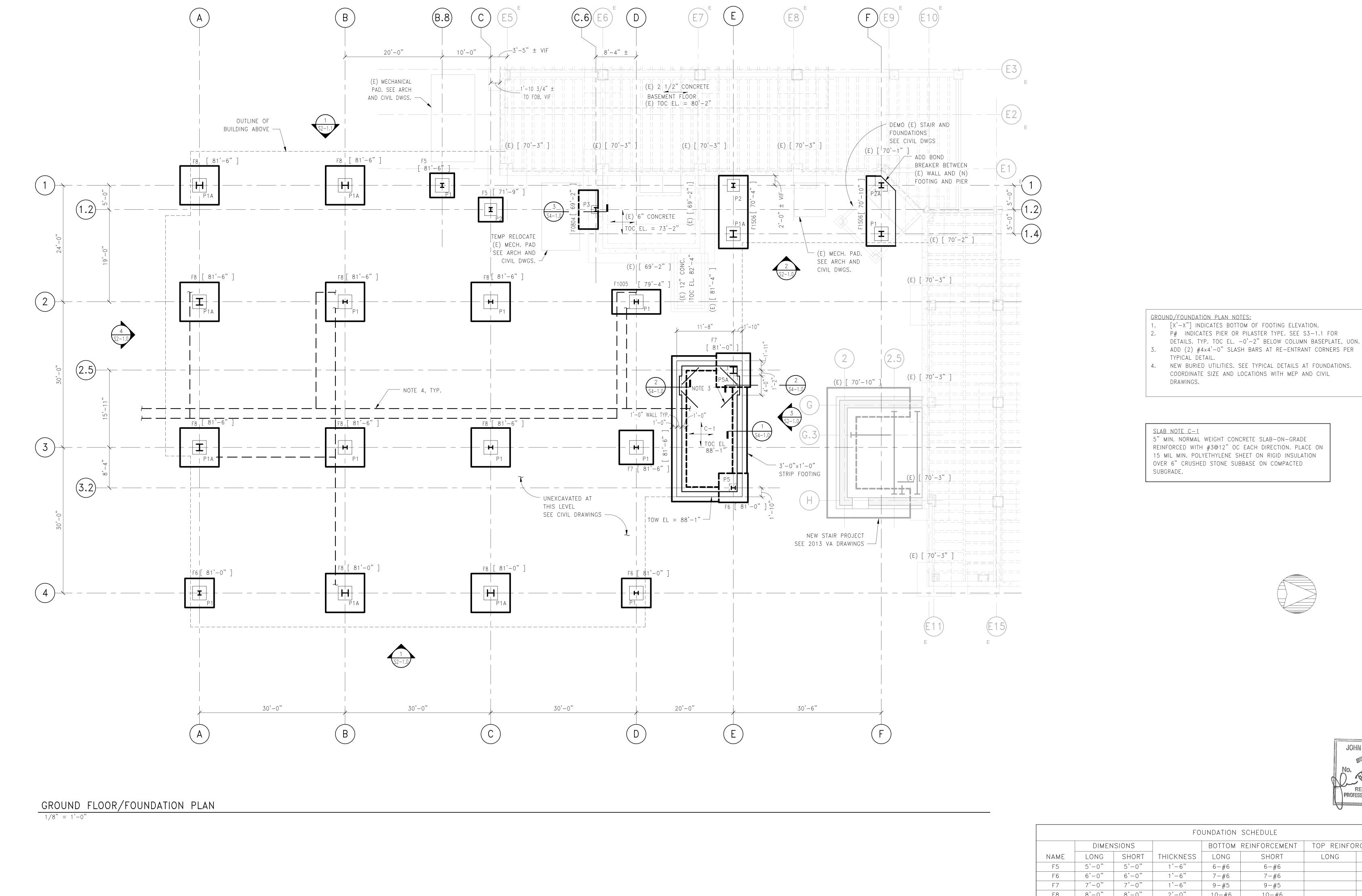




ISSUED FOR CONSTRUCTION DOCUMENTS **FULLY SPRINKLERED**

CONSULTANTS				ARCHITECT			TYPICAL STEEL DETAILS	Project Title REPLACE/I	RELOCATE	Date 2/27/2015	1,0°
CIVIL STRU PARE Corporation Simple	RUCTURAL pson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	COST D.G. Jones	morrisms witzer environments for health				DEFICIENT	ICU	VA Project No. 650-324	
8 Blackstone Valley Place 41 Se	Seyon Street	525 Mill Street Marion, MA 02738	3 Baldwin Green Common Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210			Drawn By: CMB	Building #	Scale	Drawing No.	ar tm
	81.907.9000 v.sgh.com	T: 508.748.2620 www.thompson-consulting.com	T: 781.932.3131	T: 617.772.0260 F: 617.772.0261 www.morrisswitzer.com	EY PLAN	N	Checked By: KAK/MJB Approved By: JHT	Location PROVIDENCE, RI		S0-2.2	OSF, Ce





	DIMEN	SIONS		BOTTOM REINFORCEMENT			TOP REINFORCEMENT		
NAME	LONG	SHORT	THICKNESS	LONG	SHORT	LONG	SHORT	NOTES	
F5	5'-0"	5'-0"	1'-6"	6-#6	6-#6				
F6	6'-0"	6'-0"	1'-6"	7-#6	7-#6				
F7	7'-0"	7'-0"	1'-6"	9-#5	9-#5				
F8	8'-0"	8'-0"	2'-0"	10-#6	10-#6				
F0804	8'-0"	4'-0"	1'-6"	6-#6	9-#5				
F1005	10'-0"	5'-0"	1'-6"	8-#7	11-#6				
F1506	15'-0"	6'-0"	2'-0"	7-#7	14-#7	7-#7	14-#7		

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	CONSULTANTS CIVIL PARE Corporation	STRUCTURAL Simpson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	COST D.G. Jones	ARCHITECT morrisms witzer environments for health			GROUND/FOUNDATION PLAN	Project Title REPLACE/R DEFICIENT		Date 2/27/2015 VA Project No. 650-324	
	8 Blackstone Valley Place Lincoln, RI 02865	41 Seyon Street Waltham, MA 02453	525 Mill Street Marion, MA 02738	3 Baldwin Green Common Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210			Drawn By: CMB	Building #	Scale	Drawing No.	
	T: 401.334.4100 www.parecorp.com	T: 781.907.9000 www.sgh.com	T: 508.748.2620 www.thompson-consulting.com	T: 781.932.3131	T: 617.772.0260 F: 617.772.0261		N	Checked By: KAK/MJB	Location	1/8" = 1'-0"	S1-2.0	
mber Date Revision	www.parecorp.com	www.agn.com	www.mompson-consulting.com	www.agjonesboston.com	www.morrisswitzer.com	KEY PLAN		Approved By: JHT	PROVIDENCE, RI			

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20'-0" 21'-8" $|8'-4''| \pm$ L3x2-1/2x5/16 W12x16 (10) V=83k -W18x35 (18)|W12x26(16)|W18x35 (16) V=90k − V=92k 1'-2 1/2" M=220k−ft — W14x22 (10) $W16 \times 26 (16) C = 3/4$ " W18x35 (18) (2)-#6 SLASH W18x40 (20) W18x35 (20) ≥ W18x40 (20) W16x31 (16) C=3/4" $W16\times31 (16) C=3/4$ " W14x22 (10) $W16\times31 (16) C=3/4"$ M=170k-ftW18x40 (28) C=3/4" W14x22 (12) W18x50 (26) $W18 \times 35 (16) C = 3/4$ " -1'-2 1/2" EOS W10x15 (8) W14x22 (12) $W18 \times 35 (16) C = 3/4$ " $W18 \times 35 (16) C = 3/4$ " NOTE 4 $W16\times36$ (18) C=3/4" W14x22 (12) $W18 \times 35 (16) C = 3/4$ " $W18 \times 35 (16) C = 3/4$ " W14x22 (12) 1'-0 1/2" $W18 \times 35 (18) C = 3/4$ " EOS $W18 \times 35$ (16) C = 3/4" $W18 \times 35 (16) C = 3/4$ " W18x35 (16) C=3/4" $\overline{\text{W18}\times\text{35}}$ (16) $\overline{\text{C}}=\overline{\text{3/4}}$ " W18x35+PLATE $W18 \times 35$ (16) C = 3/4" $W18 \times 35 (16) C = 3/4$ " $W18 \times 35 (16) C = 3/4$ " 11'-0" 1'-2 1/2' EOS 1'-2 1/2" EOS V=115k V=83k W18x35 (18) W27x94 (30) W18x35 (18) ─ NOTE 3, TYP. └─ SILL SUPPORT FRAME, TYP. L3x2-1/2x5/16 — SEE ELEVATIONS L3x2-1/2x5/1630'-0" 30'-0" 30'-0" 20'-0" 30'-6" SECOND FLOOR FRAMING PLAN 1/8" = 1'-0"

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SECOND FLOOR PLAN NOTES:

1. SEE ARCH. DWGS FOR EOS DIMS.

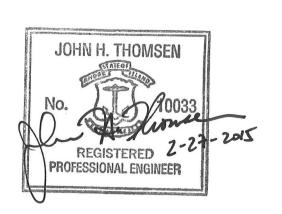
- 2. SEE ARCH. DWGS FOR STAIR DETAILS, TYP. USE FULL DEPTH STIFFENER PLATE CONNECTION FOR ALL BEAMS FRAMING INTO PERIMETER EDGE BEAMS, SEE
- TYPICAL DETAIL. 4. COORDINATE ALL KNOCKOUT SHAFT DETAILS AND EOS DIMS WITH ARCH. DWGS. RUN DECK CONTINUOUS PER
- 5. 2-1/2" GAP FOR 1-1/2" MOVEMENT JOINT. SEE ARCH.

DRAWINGS JOINT DETAIL.

SLAB NOTE S-1

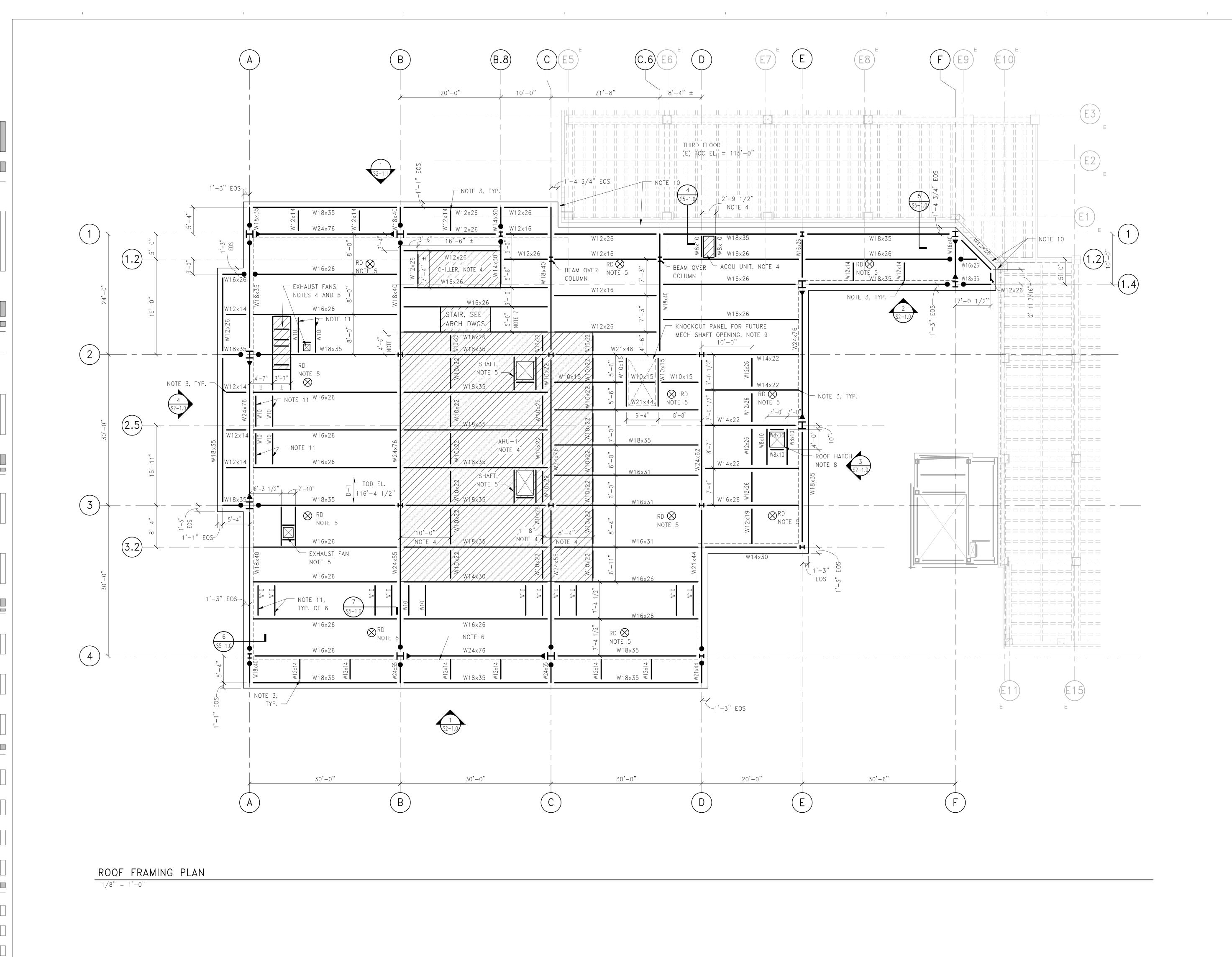
4 1/2" NORMAL WEIGHT CONCRETE SLAB ON 3"-20GA. COMPOSITE METAL DECK (TOTAL DEPTH = 7-1/2"). REINFORCE WITH #3@12" OC EACH WAY. SEE PLAN FOR TOC EL. TOS EL. = 7-1/2" BELOW TOC UON WITH $[\pm X'-X"]$ INDICATING CHANGE FROM TYPICAL TOS EL.





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SLAB NOTE D-1 3", 20 GA. GALV METAL ROOF DECK TYPE B. TYPICAL TOS EL. = 3" BELOW DECK EL. NOTED ON

ROOF PLAN NOTES:

1. SEE ARCH. DWGS FOR EOS.

2. SEE ARCH. DWGS FOR STAIR DETAILS. 3. USE FULL DEPTH STIFFENER PLATE CONNECTION FOR ALL BEAMS FRAMING INTO PERIMETER EDGE BEAM. SEE TYPICAL DETAILS.

4. CONTRACTOR TO COORDINATE EQUIPMENT SUPPORTS AND OPENING FRAMING LOCATIONS WITH APPROVED ROOFTOP EQUIPMENT SUBMITTALS, ARCH. AND MEP-FP DRAWINGS. AHU CURBS TO BEAR DIRECTLY ON ROOF STEEL BEAMS. CENTER STEEL FRAMING UNDER EQUIPMENT SUPPORTS, UON. 5. SEE TYPICAL DETAILS FOR FRAMING AT ROOF DRAINS AND

VENTS OPENINGS. MAXIMUM DRAIN OR VENT EQUIPMENT WEIGHT = 600LBS. COORDINATE LOCATION WITH ARCH. AND MEP DRAWINGS.

6. PROVIDE GUARDRAIL PER ARCH DETAILS, COORD EXTENTS W/ ARCH. DWGS. 7. COORDINATE EXTERIOR STAIR SUPPORT BEAM LOCATION WITH

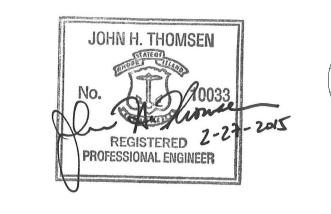
ARCH. DWGS. 8. COORDINATE ROOF HATCH SIZE AND LOCATION WITH ARCH. DRAWINGS.

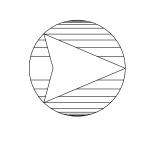
9. COORDINATE FUTURE ROOF SHAFT LOCATIONS WITH ARCH. DRAWINGS. DO NOT CUT METAL DECK AT THIS TIME. 10. 3" GAP MIN. FOR 2" MOVEMENT JOINT. SEE ARCH. DRAWINGS

FOR JOINT DETAILS. 11. EQUIPMENT BOOM SUPPORTS, SEE SECOND FLOOR REFLECTED CEILING PLAN SHEET S1-3.0.

EQUIPMENT DESIGN PARAMETERS SET ALL EQUIPMENT ON PRE-FABRICATED CURBS ON METAL DECK. DESIGN WEIGHT PLAN DIMS NAME

50'x40' AHU-1160 KIPS 7'-4"x16'-6" CHILLER 8 KIPS ACCU 5'x3' 1 KIP



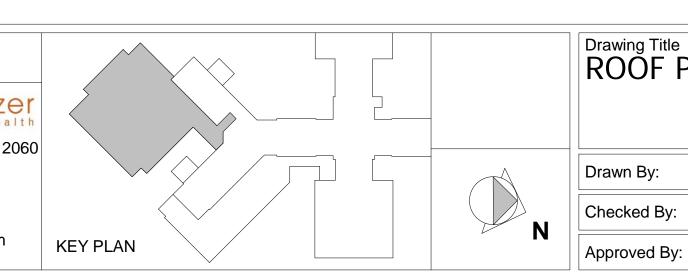


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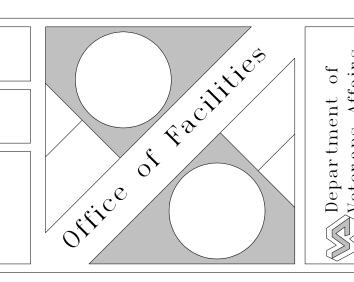


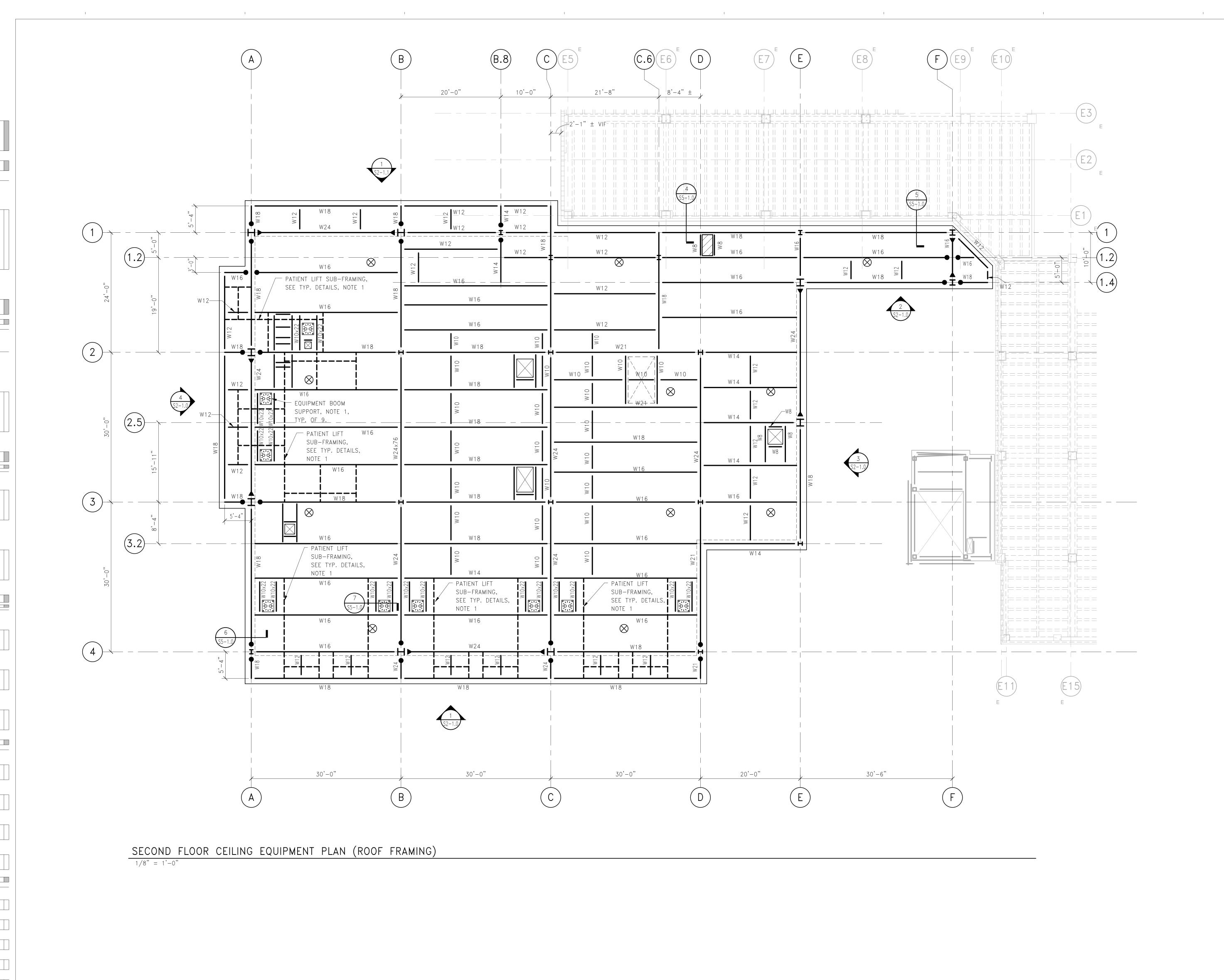
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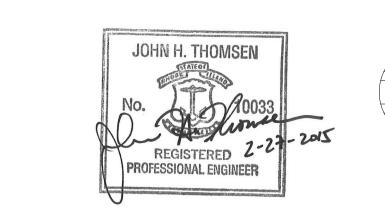


PLAN	Project Title REPLACE/R DEFICIENT	Date 2/27/2015 VA Project No. 650-324		
СМВ	Building #	Scale	Drawing No.	
y: KAK/MJB	Location	1/8" = 1'-0"	S1-2.	
Ву: ЈНТ	PROVIDENCE, RI			





SECOND FLOOR RCP NOTES: 1. CONTRACTOR MUST COORDINATE FINAL EQUIPMENT SUPPORT LOCATIONS WITH APPROVED MANUFACTURER'S SUBMITTAL AND DESIGN SUPPORTS FOR OWNER SELECTED EQUIPMENT.

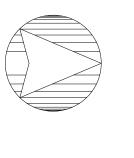


2/27/2015

650-324

VA Project No.

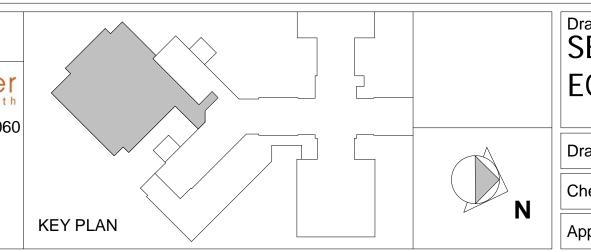
Drawing No.



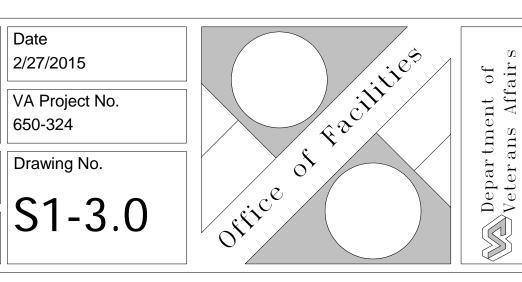
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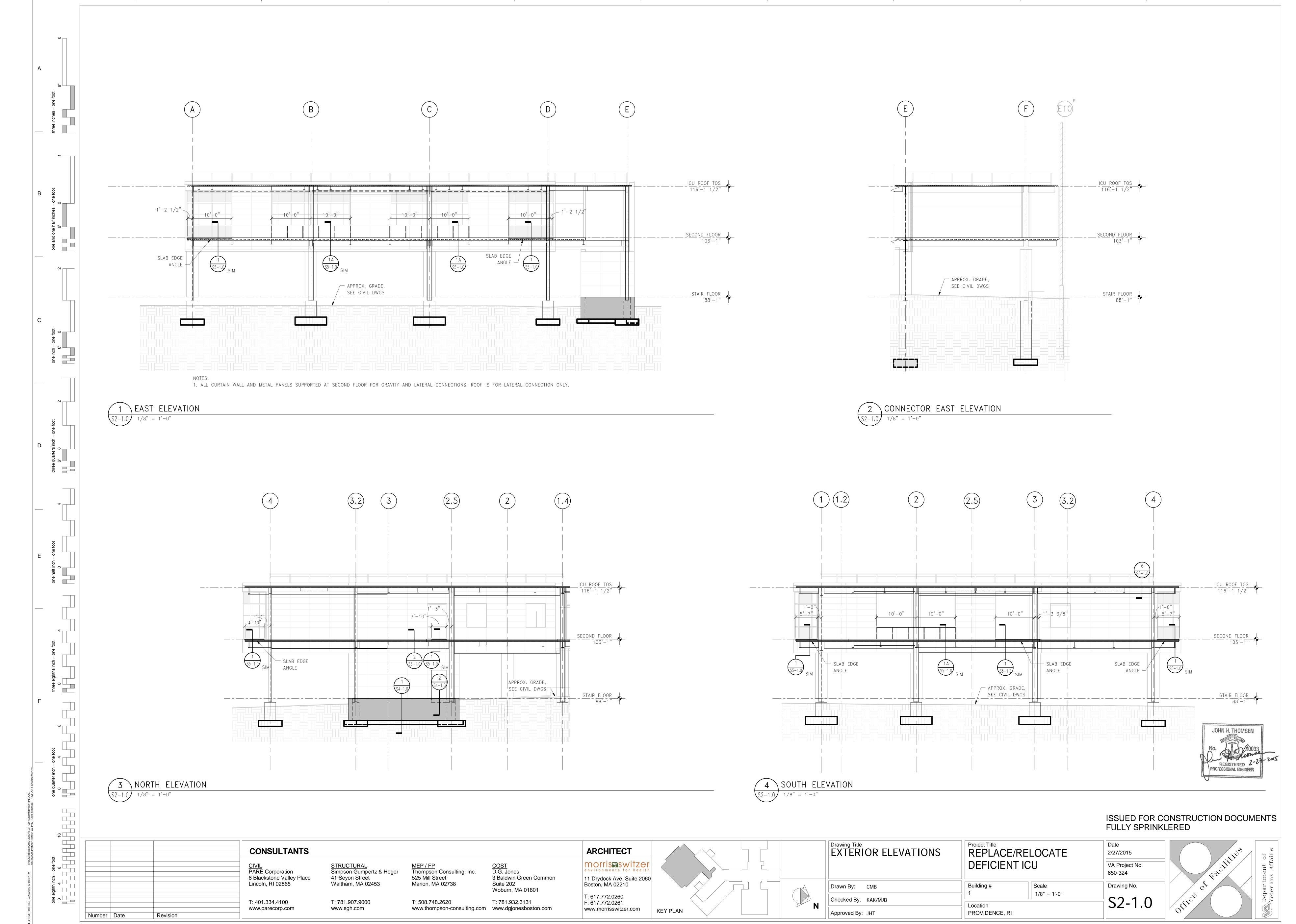
		CONSULTANTS				ARCHITECT
		CIVIL PARE Corporation 8 Blackstone Valley Place Lincoln, RI 02865	STRUCTURAL Simpson Gumpertz & Heger 41 Seyon Street Waltham, MA 02453	MEP / FP Thompson Consulting, Inc. 525 Mill Street Marion, MA 02738	COST D.G. Jones 3 Baldwin Green Common Suite 202 Woburn, MA 01801	norrisms witzer environments for health 11 Drydock Ave, Suite 2060 Boston, MA 02210
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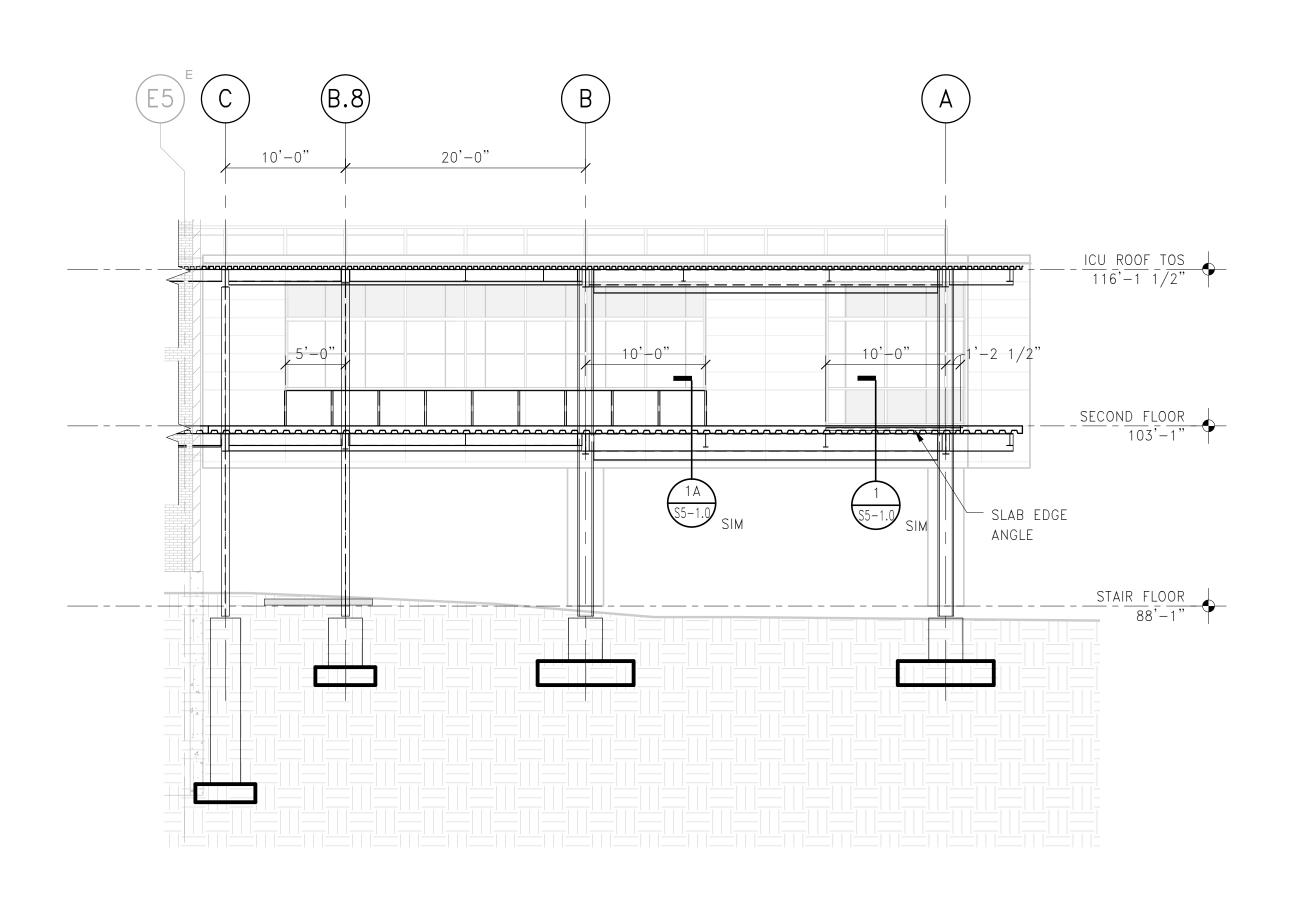


SECOND FLOOR CEILING EQUIPMENT PLAN	REPLACE/RE DEFICIENT IO			
Drawn By: CMB	Building #	Scale		
Checked By: KAK/MJB	Location	1/8" = 1'-0"		
Approved By: JHT	PROVIDENCE, RI			



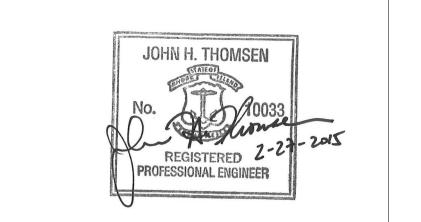


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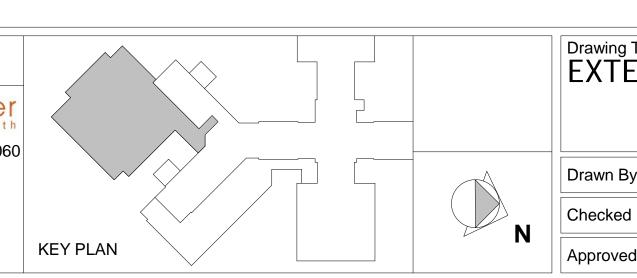
WEST ELEVATION

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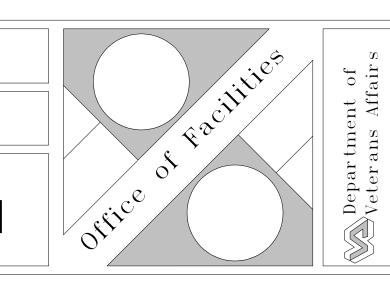


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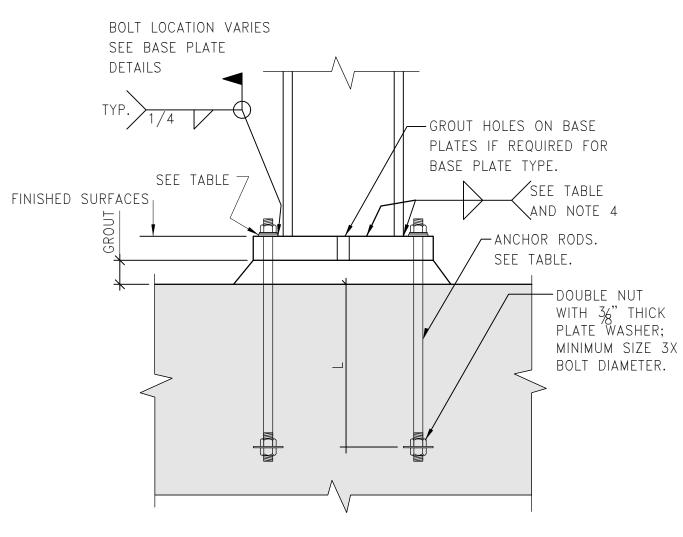
		CONSULTANTS				ARCHITECT
		<u>CIVIL</u> PARE Corporation	STRUCTURAL Simpson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	<u>COST</u> D.G. Jones	morrisvswitzer
		8 Blackstone Valley Place Lincoln, RI 02865	41 Seyon Street Waltham, MA 02453	525 Mill Street Marion, MA 02738	3 Baldwin Green Common Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210
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Number	Date Revision					



ERIOR ELEVATIONS	Project Title REPLACE/RE DEFICIENT IC	Date 2/27/2015 VA Project No. 650-324		
By: CMB	Building #	Scale	Drawing No.	
ed By: KAK/MJB	Location	1/8" = 1'-0"	S2-1.1	
ed By: JHT	PROVIDENCE, RI			



COLUMN SCHEDULE STAIR ROOF TOS 130'-1" ICU ROOF TOS 116'-1 1/2" SECOND FLOOR 103'-1" STAIR FLOOR 88'-1" BOTTOM OF BASE $87'-3" \quad 87'-3" \quad 8$ PLATE ELEVATION BASE PLATE TYPE BP-3 | BP-3 | BP-1 | BP-3 | BP-1 | BP-1 | BP-3 | BP-1 | BP-1 | BP-1 | BP-1 | BP-1 | BP-1 | BP-2 | BP-2 | BP-2 BASE PLATE 3/4" | 3/4" | 1 1/2" | 1 1/2" | 3/4" | 1 1/2" | 1 1/2" | 1 1/2" | 3/4" | 1 1/2" | 1" 1" | 1 1/2" | 3/4" | 1" | 1" | 1" | 1 1/2" | 1" THICKNESS P1A P1A P1 P1A P1 P1 P1 P1 P1 P2 P1 P1 P1 P1 P2 P1 P1 P1 P1 P2 P1A P5A P5 P2A P1 PILASTER TYPE NOTES MF COLUMN LOCATIONS COLUMN SCHEDULE NOTES COLUMN SCHEDULE NOTES COLUMN SCHEDULE SYMBOLS LEGEND COLUMN SCHEDULE SYMBOLS LEGEND (CONT.) 1. TOP OF STANDARD COLUMN STACK ELEVATION IS AT TOP OF MF INDICATES THAT COLUMN IS PART OF MOMENT FRAME INDICATES COLUMN SPLICE WITH END CAP PLATE AND BASE LATERAL LOAD-RESISTING SYSTEM. STEEL ELEVATION OF HIGHEST BEAM FRAMING TO IT UON. TOP OF COLUMNS WITH CAP PLATES SHALL ALIGN WITH TOP INDICATES TYPICAL GRAVITY COLUMN SPLICE. INDICATES THAT COLUMN LOCATION IS OFFSET IN PLAN FROM GRID. SEE PLAN FOR EXACT LOCATIONS. OF CONCRETE SLAB. 2. SEE BASE PLATE DETAILS ON S3-1.0 FOR INFORMATION ON INDICATES THAT COLUMN BEARS ON STEEL TRANSFER BEAM. BASE PLATE TYPES. SEE TYPICAL DETAIL. 3. SEE CONCRETE PILASTER DETAILS ON S3-1.1 FOR INFORMATION INDICATES BEAM FRAMES OVER COLUMN WITH CAP PLATE. ON PILASTER TYPES. SEE TYPICAL DETAIL. 4. WHERE INDICATED BY "GALV.", COLUMN SHALL BE HOT-DIPPED GALVANIZED. INDICATES THAT COLUMN BEARS ON CONCRETE PILASTER. 5. IF FABRICATOR ADDS COLUMNS SPLICES FOR ERECTION, PLACE ALL SPLICES +4'-6" FROM NEAREST LEVEL. INDICATES THAT COLUMN BEARS ON CONCRETE WALL. BOTH FLANGES 5/16 5/16 \Diamond \circ \circ BASE PLATE TYPE: BP-1 (W10 GRAVITY) 1/2" = 1'-0"

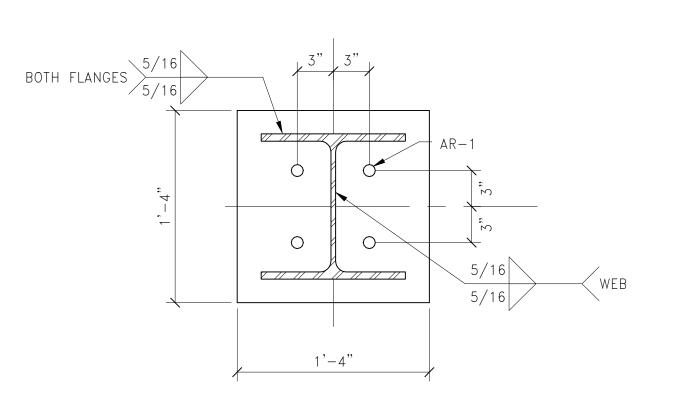


1. PROVIDE LEVELING NUTS AND SHIM PACKS AS REQUIRED TO MAINTAIN COLUMN LEVELNESS.

- 2. WASHER PLATES ASTM A36 3. MATCH SURFACE PREPARATION AND FINISH OF BEARING
- PLATE, ANCHOR RODS, WASHERS, AND WASHER PLATES TO THAT OF COLUMNS. 4. FOR ROUND COLUMNS, WELD ALL AROUND PERIMETER OF SECTION.

TVDE	ANCHOR		DOD MATERIAL	NUT	WASHER	WASHER	MINI WELD	CDOUT	BASE PL	NOTES	
TYPE	DIAMETER	L	ROD MATERIAL	MATERIAL	DIAMETER	THICKNESS	MIN. WELD	GROUT	ANCHOR ROD HOLE		
AR-1	3/4"ø	1'-6"	F1554 GRADE 55	A563 GRADE A	2"ø	3/8"	5/16	2"	1 5/16"ø	WELDABILITY SUPPLEMENT	
AR-2	1 1/4"ø	2'-0"	F1554 GRADE 55	A563 GRADE A	3"ø	1/2"	5/16	2"	2 1/16"ø	WELDABILITY SUPPLEMENT	







STAIR ROOF TOS

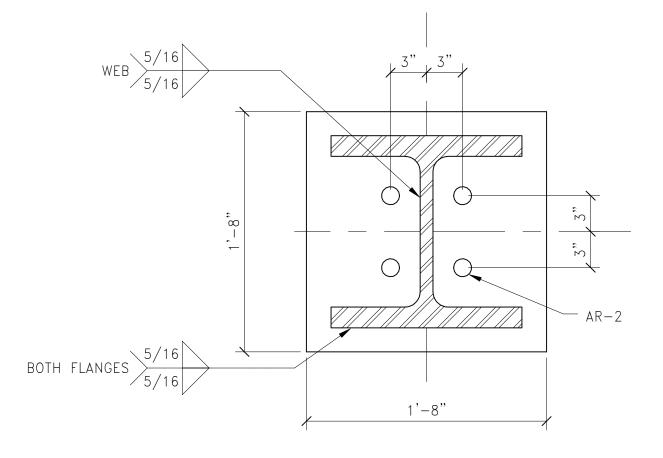
ICU ROOF TOS

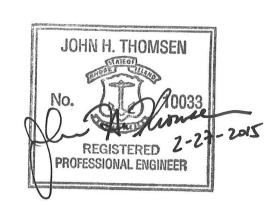
116'-1 1/2"

SECOND FLOOR

MF

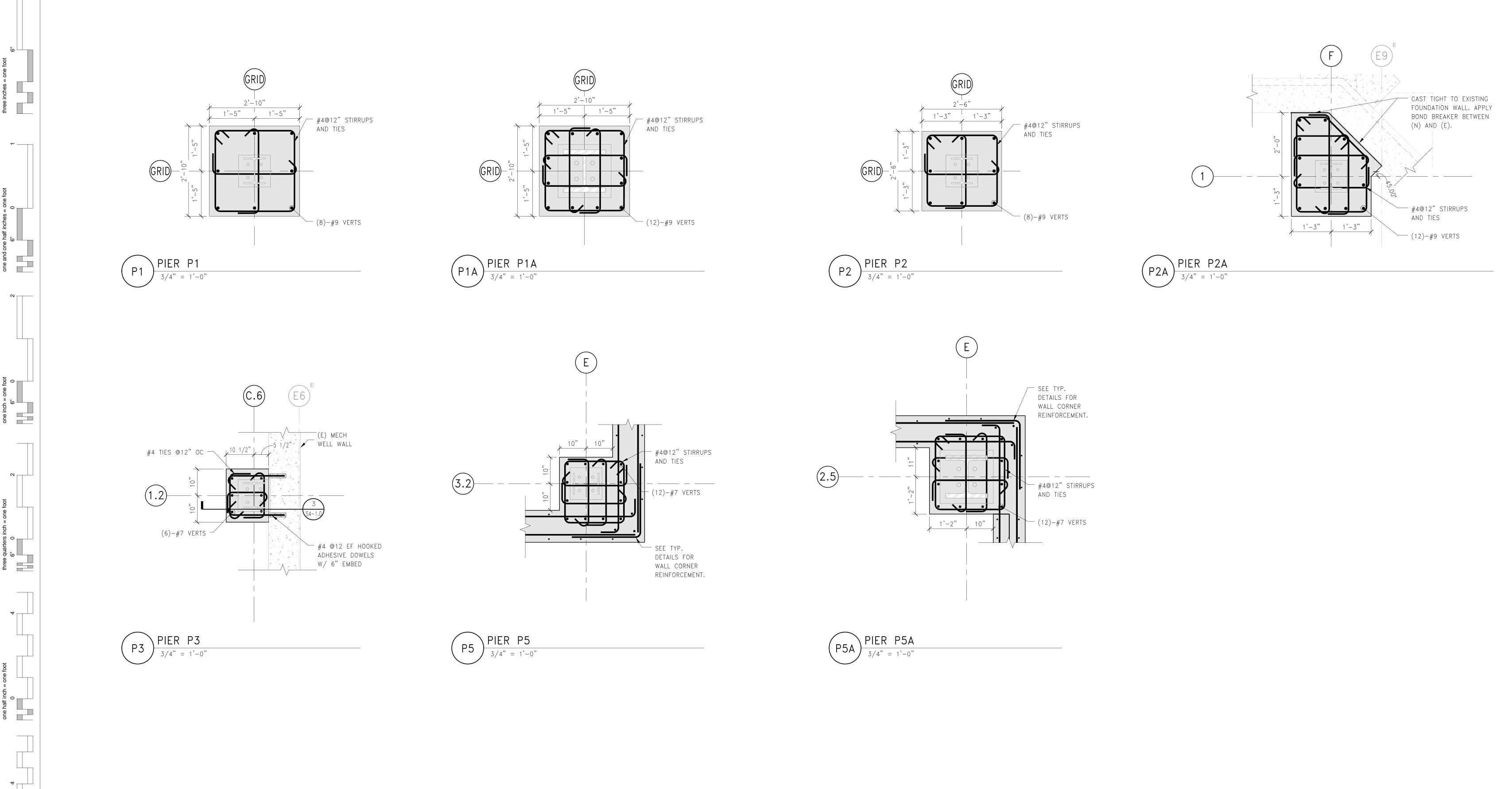
130'-1"

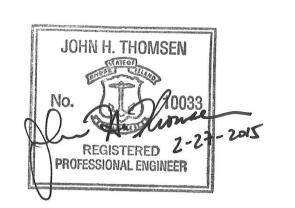




ISSUED FOR CONSTRUCTION DOCUMENTS FULLY SPRINKLERED

	CONSULTANTS							Drawing Title COLUMN SCHEDULE AND	Project Title REPLACE/RELOCATE		Date 2/27/2015	ir s
	CIVIL PARE Corporation 8 Blackstone Valley Place	STRUCTURAL Simpson Gumpertz & Heger 41 Seyon Street	MEP / FP Thompson Consulting, Inc. 525 Mill Street	COST D.G. Jones 3 Baldwin Green Common	morrisvswitzer environments for health 11 Drydock Ave, Suite 2060			BASEPLATE DETAILS	DEFICIENT	ICU	VA Project No. 650-324	ment of the state
	Lincoln, RI 02865	Waltham, MA 02453	Marion, MA 02738	Suite 202 Woburn, MA 01801	Boston, MA 02210 T: 617.772.0260			Drawn By: CMB	Building #	Scale As indicated	Drawing No.	epar tu
Number Date Revision	T: 401.334.4100 www.parecorp.com	T: 781.907.9000 www.sgh.com	T: 508.748.2620 www.thompson-consulting.com	T: 781.932.3131 www.dgjonesboston.com	F: 617.772.0261	EY PLAN	N	Checked By: KAK/MJB Approved By: JHT	Location PROVIDENCE, RI		S3-1.0	Office Office

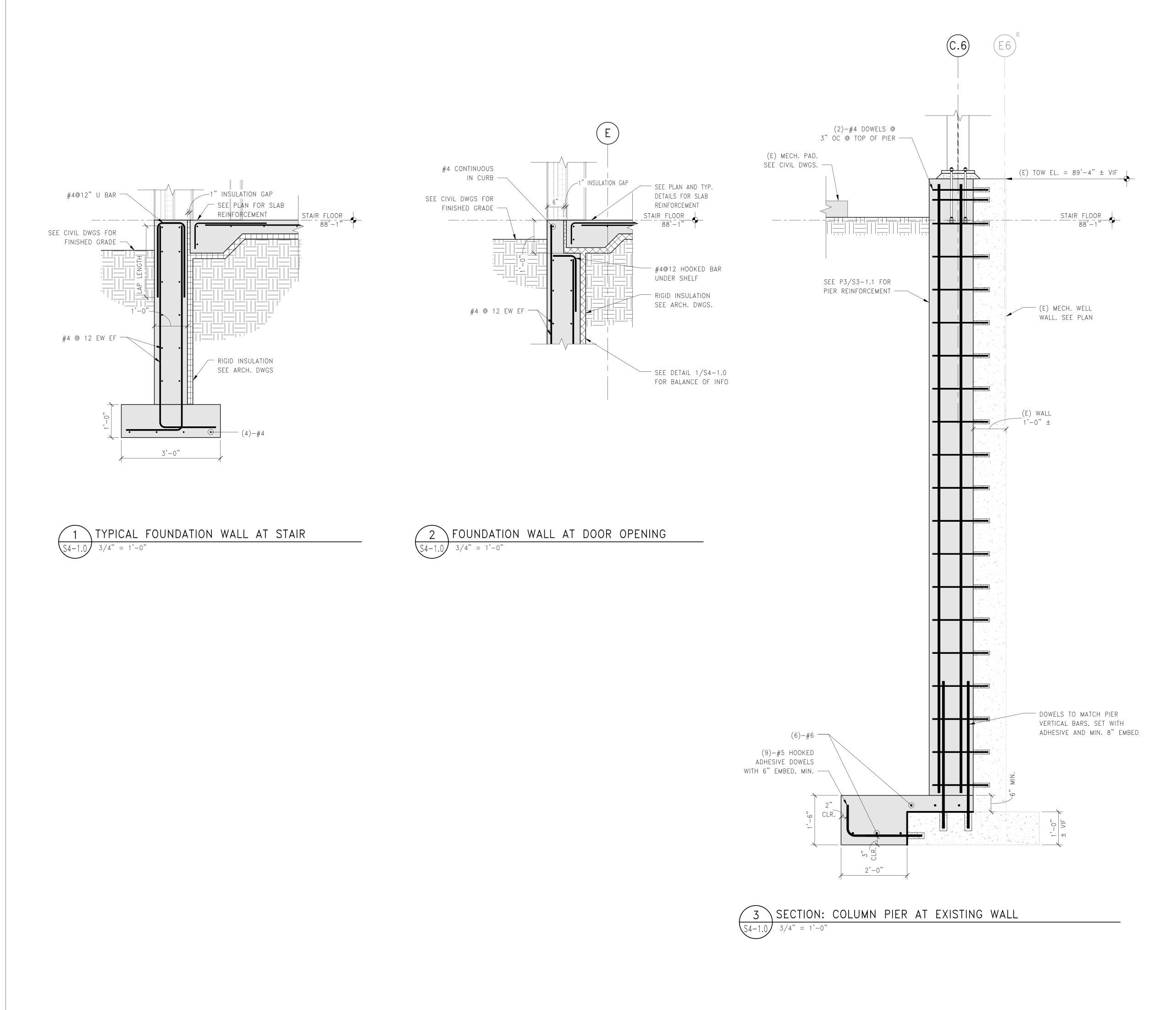


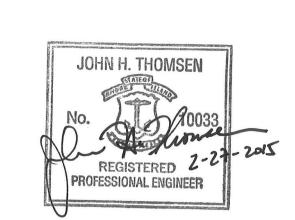


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										1 OLL 1 OF KINK	
	CONSULTANTS				ARCHITECT			PIER DETAILS	Project Title REPLACE/RELOCATE	Date 2/27/2015	
	CIVIL PARE Corporation 8 Blackstone Valley Place	STRUCTURAL Simpson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	COST D.G. Jones 3 Baldwin Green Common	morrisms witzer environments for health				DEFICIENT ICU	VA Project No. 650-324	
	Lincoln, RI 02865	41 Seyon Street Waltham, MA 02453	525 Mill Street Marion, MA 02738	Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210			Drawn By: CMB	Building # Scale 1 3/4" = 1'-0"	Drawing No.	
	T: 401.334.4100 www.parecorp.com	T: 781.907.9000 www.sgh.com	T: 508.748.2620 www.thompson-consulting.com	T: 781.932.3131 www.dgjonesboston.com	T: 617.772.0260 F: 617.772.0261 www.morrisswitzer.com	KEY PLAN	N	Checked By: KAK/MJB	Location	S3-1.1	
Number Date Revision						IXL I I LAIN		Approved By: JHT	PROVIDENCE, RI		

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2/27/2015

650-324

Drawing No.

VA Project No.

	CONSULTANTS				ARCHITECT			Drawing Title CONCRETE SECTIONS	Project Title REPLACE/RELOCATE	
	CIVIL PARE Corporation	STRUCTURAL Simpson Gumpertz & Heger	MEP / FP Thompson Consulting, Inc.	COST D.G. Jones	morrisms witzer			AND DETAILS	DEFICIENT IC	CU
	8 Blackstone Valley Place Lincoln, RI 02865	41 Seyon Street 525		3 Baldwin Green Common Suite 202 Woburn, MA 01801	11 Drydock Ave, Suite 2060 Boston, MA 02210			Drawn By: CMB	Building #	Scale
	T: 401.334.4100	T: 781.907.9000	T: 508.748.2620	T: 781.932.3131	T: 617.772.0260 F: 617.772.0261		N	Checked By: KAK/MJB	Location	3/4" = 1'-0"
Number Date Revision	www.parecorp.com	www.sgh.com	www.thompson-consulting.com	www.dgjonesboston.com	www.morrisswitzer.com	KEY PLAN	•	Approved By: JHT	PROVIDENCE, RI	

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